

AD-A107 689

BLACK AND VEATCH KANSAS CITY MO
NATIONAL DAM SAFETY PROGRAM. BURTON-DUENKE DAM NUMBER 4 (MO 317--ETC(U)
APR 81 E R BURTON, H L CALLAHAN

DACW43-81-C-0037

F/G 13/13

317--ETC(U)

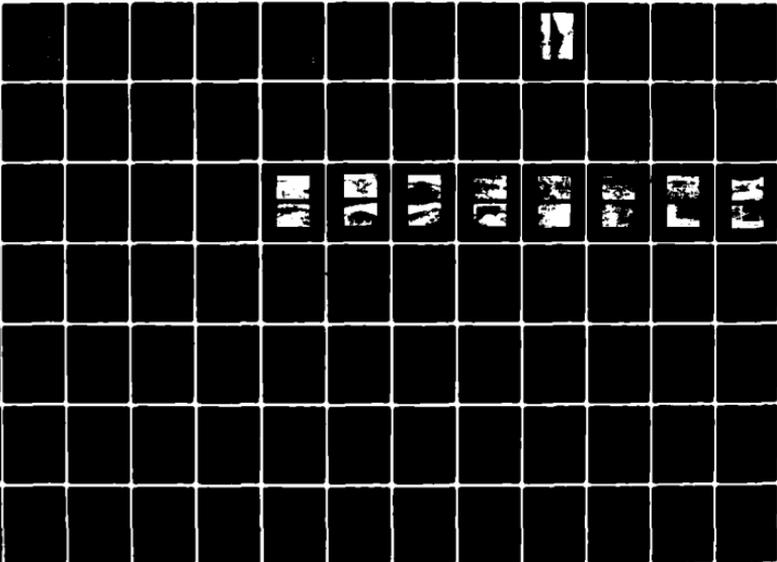
NL

UNCLASSIFIED

1 of 2

ADA

07000



LEVEL II

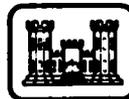
D

MISSOURI-OSAGE-GASCONADE BASIN

AD A I U 7 689

**BURTON-DUENKE DAM #4
CAMDEN COUNTY, MISSOURI
MO 31713**

**PHASE 1 INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM**



**United States Army
Corps of Engineers**
*... Serving the Army
... Serving the Nation*

St. Louis District

DTIC
ELECTE
NOV 20 1981
S **D**
D

DIG FILE COPY

PREPARED BY: U.S. ARMY ENGINEER DISTRICT. ST. LOUIS

FOR: STATE OF MISSOURI

APRIL 1981

81 11 19 002

DISTRIBUTION STATEMENT A

**Approved for public release;
Distribution Unlimited**

UNCLASSIFIED

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER	2. GOVT ACCESSION NO. AD-A107	3. RECIPIENT'S CATALOG NUMBER 659
4. TITLE (and Subtitle) Phase I Dam Inspection Report National Dam Safety Program Burton-Duenke Lake No. 4 (MO 31713) Camden County, Missouri		5. TYPE OF REPORT & PERIOD COVERED Final Report
		6. PERFORMING ORG. REPORT NUMBER
7. AUTHOR(s) Black & Veatch, Consulting Engineers		8. CONTRACT OR GRANT NUMBER(s) DACW43-81-C-0037
9. PERFORMING ORGANIZATION NAME AND ADDRESS U.S. Army Engineer District, St. Louis Dam Inventory and Inspection Section, LMSED-PD 210 Tucker Blvd., North, St. Louis, Mo. 63101		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS
11. CONTROLLING OFFICE NAME AND ADDRESS U.S. Army Engineer District, St. Louis Dam Inventory and Inspection Section, LMSED-PD 210 Tucker Blvd., North, St. Louis, Mo. 63101		12. REPORT DATE April 1981
		13. NUMBER OF PAGES Approximately 95
14. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office)		15. SECURITY CLASS. (of this report) UNCLASSIFIED
		15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
16. DISTRIBUTION STATEMENT (of this Report) Approved for release; distribution unlimited.		
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)		
18. SUPPLEMENTARY NOTES		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number) Dam Safety, Lake, Dam Inspection, Private Dams		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) This report was prepared under the National Program of Inspection of Non-Federal Dams. This report assesses the general condition of the dam with respect to safety, based on available data and on visual inspection, to determine if the dam poses hazards to human life or property.		

DD FORM 1 JAN 73 1473 EDITION OF 1 NOV 65 IS OBSOLETE

UNCLASSIFIED
SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

INSTRUCTIONS FOR PREPARATION OF REPORT DOCUMENTATION PAGE

RESPONSIBILITY. The controlling DoD office will be responsible for completion of the Report Documentation Page, DD Form 1473, in all technical reports prepared by or for DoD organizations.

CLASSIFICATION. Since this Report Documentation Page, DD Form 1473, is used in preparing announcements, bibliographies, and data banks, it should be unclassified if possible. If a classification is required, identify the classified items on the page by the appropriate symbol.

COMPLETION GUIDE

Block 0. General. Make Blocks 1, 4, 5, 6, 7, 11, 13, 15, and 16 agree with the corresponding information on the report cover. Leave Blocks 2, 3, 8, 9, 10, 12, 14, 17, 18, and 19 blank.

Block 1. Report Number. Enter the unique alphanumeric report number shown on the cover.

Block 2. Government Accession No. Leave Blank. This space is for use by the Defense Documentation Center.

Block 3. Recipient's Catalog Number. Leave blank. This space is for the use of the report recipient to assist in future retrieval of the document.

Block 4. Title and Subtitle. Enter the title in all capital letters exactly as it appears on the publication. Titles should be unclassified whenever possible. Write out the English equivalent for Greek letters and mathematical symbols in the title (see "Abstracting Scientific and Technical Reports of Defense-sponsored RDT/E," AD-667 000). If the report has a subtitle, this subtitle should follow the main title, be separated by a comma or semicolon if appropriate, and be initially capitalized. If a publication has a title in a foreign language, translate the title into English and follow the English translation with the title in the original language. Make every effort to simplify the title before publication.

Block 5. Type of Report and Period Covered. Indicate here whether report is interim, final, etc., and, if applicable, inclusive dates of period covered, such as the life of a contract covered in a final contractor report.

Block 6. Performing Organization Report Number. Only numbers other than the official report number shown in Block 1, such as series numbers for in-house reports or a contractor/grantee number assigned by him, will be placed in this space. If no such numbers are used, leave this space blank.

Block 7. Author(s). Include corresponding information from the report cover. Give the name(s) of the author(s) in conventional order (for example, John R. Doe or, if author prefers, J. Robert Doe). In addition, list the affiliation of an author if it differs from that of the performing organization.

Block 8. Contract or Grant Number(s). For a contractor or grantee report, enter the complete contract or grant number(s) under which the work reported was accomplished. Leave blank in in-house reports.

Block 9. Performing Organization Name and Address. For in-house reports enter the name and address, including office symbol, of the performing activity. For contractor or grantee reports enter the name and address of the contractor or grantee who prepared the report and identify the appropriate corporate division, school, laboratory, etc., of the author. List city, state, and ZIP Code.

Block 10. Program Element, Project, Task Area, and Work Unit Numbers. Enter here the number code from the applicable Department of Defense form, such as the DD Form 1498, "Research and Technology Work Unit Summary" or the DD Form 1634, "Research and Development Planning Summary," which identifies the program element, project, task area, and work unit or equivalent under which the work was authorized.

Block 11. Controlling Office Name and Address. Enter the full, official name and address, including office symbol, of the controlling office. (Equates to funding/sponsoring agency. For definition see DoD Directive 5200.20, "Distribution Statements on Technical Documents.")

Block 12. Report Date. Enter here the day, month, and year or month and year as shown on the cover.

Block 13. Number of Pages. Enter the total number of pages.

Block 14. Monitoring Agency Name and Address (if different from Controlling Office). For use when the controlling or funding office does not directly administer a project, contract, or grant, but delegates the administrative responsibility to another organization.

Blocks 15 & 15a. Security Classification of the Report: Declassification/Downgrading Schedule of the Report. Enter in 15 the highest classification of the report. If appropriate, enter in 15a the declassification/downgrading schedule of the report, using the abbreviations for declassification/downgrading schedules listed in paragraph 4-207 of DoD 5200.1-R.

Block 16. Distribution Statement of the Report. Insert here the applicable distribution statement of the report from DoD Directive 5200.20, "Distribution Statements on Technical Documents."

Block 17. Distribution Statement (of the abstract entered in Block 20, if different from the distribution statement of the report). Insert here the applicable distribution statement of the abstract from DoD Directive 5200.20, "Distribution Statements on Technical Documents."

Block 18. Supplementary Notes. Enter information not included elsewhere but useful, such as: Prepared in cooperation with . . . Translation of (or by) . . . Presented at conference of . . . To be published in . . .

Block 19. Key Words. Select terms or short phrases that identify the principal subjects covered in the report, and are suitably specific and precise to be used as index entries for cataloging, conforming to standard terminology. The DoD "Thesaurus of Engineering and Scientific Terms" (TEST), AD-672 000, can be helpful.

Block 20. Abstract. The abstract should be a brief (not to exceed 200 words) factual summary of the most significant information contained in the report. If possible, the abstract of a classified report should be unclassified and the abstract to an unclassified report should consist of publicly-releasable information. If the report contains a significant bibliography or literature survey, mention it here. For information on preparing abstracts see "Abstracting Scientific and Technical Reports of Defense-Sponsored RDT&E," AD-667 000.

MISSOURI-OSAGE-GASCONADE BASIN

BURTON-DUENKE DAM #4
CAMDEN COUNTY, MISSOURI
MO 31713

PHASE 1 INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM



**United States Army
Corps of Engineers**

*... Serving the Army
... Serving the Nation*

St. Louis District

PREPARED BY: U.S. ARMY ENGINEER DISTRICT. ST. LOUIS

FOR: STATE OF MISSOURI

APRIL 1981



DEPARTMENT OF THE ARMY
ST. LOUIS DISTRICT, CORPS OF ENGINEERS
 210 TUCKER BOULEVARD, NORTH
 ST. LOUIS, MISSOURI 63101

REPLY TO
 ATTENTION OF

SUBJECT: Dam Phase I Inspection Report

This report presents the results of field inspection and evaluation of the Burton-Duenke Dam #4 (MO 31713).

It was prepared under the National Program of Inspection of Non-Federal Dams.

This dam has been classified as unsafe, non-emergency by the St. Louis District as a result of the application of the following criteria:

- a. Spillway will not pass 50 percent of the Probable Maximum Flood without overtopping the dam.
- b. Overtopping of the dam could result in failure of the dam.
- c. Dam failure significantly increases the hazard to loss of life downstream.

SIGNED

SUBMITTED BY: _____
 Chief, Engineering Division

21 JUL 1981
 Date

APPROVED BY: _____
 Colonel, CB, Commanding

21 JUL 1981
 Date

SIGNED

Accession For	
NTIS GRA&I	<input checked="" type="checkbox"/>
DTIC TAB	<input type="checkbox"/>
Unannounced	<input type="checkbox"/>
Justification	
By _____	
Distribution/	
Availability Codes	
Dist	Avail and/or
A	Special

S DTIC ELECTE D
NGV 20 1981
D

BURTON-DUENKE DAM #4

CAMDEN COUNTY, MISSOURI

MISSOURI INVENTORY NO. 31713

PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM

PREPARED BY:

BLACK & VEATCH
CONSULTING ENGINEERS
KANSAS CITY, MISSOURI

UNDER DIRECTION OF
ST. LOUIS DISTRICT CORPS OF ENGINEERS

FOR
GOVERNOR OF MISSOURI

APRIL 1981

PHASE I REPORT

NATIONAL DAM SAFETY PROGRAM

Name of Dam	Burton-Duenke Dam #4
State Located	Missouri
County Located	Camden County
Stream	Tributary of the Lake of the Ozarks
Date of Inspection	24 April 1981

Burton-Duenke Dam #4 was inspected by a team of engineers from Black & Veatch, Consulting Engineers for the St. Louis District, Corps of Engineers. The purpose of the inspection was to make an assessment of the general condition of the dam with respect to safety, based upon available data and visual inspection, in order to determine if the dam poses hazards to human life or property.

The guidelines used in the assessment were furnished by the Department of the Army, Office of the Chief of Engineers and developed with the help of several Federal and state agencies, professional engineering organizations, and private engineers. Based on these guidelines, this dam is classified as an intermediate size dam with a high downstream hazard potential. According to the St. Louis District, Corps of Engineers, failure would threaten lives and property. The estimated damage zone extends approximately one mile downstream to the Lake of the Ozarks. Within the estimated damage zone are a marina, two dwellings, and thirteen trailers. Contents of the estimated downstream damage zone were verified by the inspection team.

Our inspection and evaluation indicates that the spillway does not meet the criteria set forth in the guidelines for a dam having the above size and hazard potential. The spillway will not pass the probable maximum flood without overtopping the dam but will pass 20 percent of the probable maximum flood. The spillway will pass the flood which has a one percent chance of occurrence in any given year (100-year flood). The spillway design flood recommended by the guidelines is the probable maximum flood. The probable maximum flood is defined as the flood discharge which may be expected from the most severe combination of critical meteorologic and hydrologic conditions which are reasonably possible in the region.

Based on visual observations, this dam appears to be in good condition. Deficiencies visually observed by the inspection team were seepage at the interface of the left abutment and the downstream slope, below the inlet of the spillway pipe, and on both abutments at the upstream embankment slope, erosion on the upstream and downstream slopes, at the

embankment/abutment interfaces at each of the corners of the embankment, and below the inlet of the spillway pipe, and the very thin vegetal cover. Seepage and stability analyses required by the guidelines were not available.

There were no observed deficiencies or conditions existing at the time of the inspection which indicated an immediate safety hazard. Future corrective action and regular maintenance will be required to correct or control the described deficiencies. In addition, detailed seepage and stability analyses of the existing dam, as required by the guidelines, should be performed. A detailed report discussing each of these deficiencies is attached.

Edwin R. Burton

Edwin R. Burton, PE
Missouri E-10137

Harry L. Callahan

Harry L. Callahan, Partner
Black & Veatch



OVERVIEW OF DAM

PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM
BURTON-DUENKE DAM #4

TABLE OF CONTENTS

<u>Paragraph No.</u>	<u>Title</u>	<u>Page No.</u>
	SECTION 1 - PROJECT INFORMATION	
1.1	General	1
1.2	Description of Project	1
1.3	Pertinent Data	2
	SECTION 2 - ENGINEERING DATA	
2.1	Design	5
2.2	Construction	5
2.3	Operation	5
2.4	Geology	5
2.5	Evaluation	5
	SECTION 3 - VISUAL INSPECTION	
3.1	Findings	7
3.2	Evaluation	9
	SECTION 4 - OPERATIONAL PROCEDURES	
4.1	Procedures	10
4.2	Maintenance of Dam	10
4.3	Maintenance of Operating Facilities	10
4.4	Description of Any Warning System in Effect	10
4.5	Evaluation	10
	SECTION 5 - HYDRAULIC/HYDROLOGIC	
5.1	Evaluation of Features	11
	SECTION 6 - STRUCTURAL STABILITY	
6.1	Evaluation of Structural Stability	13
	SECTION 7 - ASSESSMENT/REMEDIAL MEASURES	
7.1	Dam Assessment	14
7.2	Remedial Measures	14

TABLE OF CONTENTS (Cont'd)

LIST OF PLATES

<u>Plate No.</u>	<u>Title</u>
1	Location Map
2	Vicinity Topography
3	Dam Plan
4	Dam Cross Section and Dam Crest Profile
5	Photo Index

LIST OF PHOTOGRAPHS

<u>Photo No.</u>	<u>Title</u>
1	Upstream Face of Dam Looking East
2	Upstream Face of Dam Looking West
3	Upstream Face of Dam at Waterline
4	Crest of Dam Looking East
5	Crest of Dam Looking West
6	Downstream Face of Dam Looking East
7	Downstream Face of Dam Looking West
8	Spillway Pipe Inlet
9	Channel Downstream of Spillway Pipe Outlet
10	Erosion on Upstream Face of Dam
11	Erosion at Upstream Face and Left Abutment
12	Erosion at Downstream Face and Left Abutment
13	Left Abutment Just Upstream of Dam

TABLE OF CONTENTS (Cont'd)

LIST OF PHOTOGRAPHS

<u>Photo No.</u>	<u>Title</u>
14	Right Bank of Reservoir
15	Lake and Watershed Viewed From Dam
16	Valley Downstream of Dam

APPENDIX

Appendix A - Hydrologic and Hydraulic Analyses

SECTION 1 - PROJECT INFORMATION

1.1 GENERAL

a. Authority. The National Dam Inspection Act, Public Law 92-367, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a program of safety inspection of dams throughout the United States. Pursuant to the above, the District Engineer of the St. Louis District, Corps of Engineers, directed that a safety inspection of the Burton-Duenke Dam #4 be made.

b. Purpose of Inspection. The purpose of the inspection was to make an assessment of the general condition of the dam with respect to safety, based upon available data and visual inspection, in order to determine if the dam poses hazards to human life or property.

c. Evaluation Criteria. Criteria used to evaluate the dam were furnished by the Department of the Army, Office of the Chief of Engineers, in "Recommended Guidelines for Safety Inspection of Dams." These guidelines were developed with the help of several Federal agencies and many state agencies, professional engineering organizations, and private engineers.

1.2 DESCRIPTION OF PROJECT

a. Description of Dam and Appurtenances.

(1) The dam is an earth structure located in the valley of a tributary to the Lake of the Ozarks (see Plate 1). The watershed is an area of steep hills consisting of about 80 percent timber, 15 percent grassland on a golf course fairway and 5 percent urban. The lake would back up to near the toe of an upstream dam when the water surface is at the spillway level. The dam is approximately 380 feet long along the crest and 44 feet high. The dam crest is 40 feet wide. The downstream face of the dam has a nonuniform slope from the crest to the valley floor below.

(2) The spillway is an uncontrolled 36-inch corrugated metal pipe installed in the embankment. The spillway has a concrete headwall at the upstream end. The pipe acts as an orifice. Flow through the pipe discharges into a ditch and then to the hillside downstream of the left abutment. There is no emergency spillway.

(3) Pertinent physical data are given in paragraph 1.3.

b. Location. The dam is located in northeast Camden County, Missouri, as indicated on Plate 1. The lake formed by the dam is in an

area shown on the United States Geological Survey 7.5 minute series quadrangle map for Lake Ozark, Missouri in Section 8 of T39N, R16W.

c. Size Classification. Criteria for determining the size classification of dams and impoundments are presented in the guidelines referenced in paragraph 1.1c above. Based on these criteria, the Burton-Duenke Dam #4, which is 44 feet high and has a normal storage capacity of 118 acre-feet, is in the intermediate size category. An intermediate size dam is classified as having a height less than 100 feet, but greater than or equal to 40 feet and/or a storage capacity less than 50,000 acre-feet, but greater than or equal to 1,000 acre-feet.

d. Hazard Classification. The hazard classification assigned by the Corps of Engineers for this dam is as follows: Burton-Duenke Dam #4 has a high hazard potential, meaning that the dam is located where failure may cause loss of life, and serious damage to homes, agricultural, industrial and commercial facilities, and to important public utilities, main highways, or railroads. For the Burton-Duenke Dam #4 the estimated flood damage zone extends approximately one mile downstream to the Lake of the Ozarks. Within the estimated damage zone are a marina, two dwellings, and thirteen trailers. Contents of the estimated downstream damage zone were verified by the inspection team.

e. Ownership. The dam is owned by the Burton-Duenke Development Company, P. O. Box 213-32, Osage Beach, Missouri 65065, c/o Mr. Westhoff.

f. Purpose of Dam. The dam forms a 9.7-acre lake used for recreation.

g. Design and Construction History. Data relating to the design and construction were not available. The owner's representative, Mr. Westhoff, stated that the dam was designed by Mr. Dave Krehbiel and was constructed in the summer of 1979.

h. Normal Operating Procedure. Normal rainfall, runoff, transpiration, evaporation, and overflow through the uncontrolled spillway all combine to maintain a relatively stable water surface elevation. The lake was considerably below normal pool at the time of the inspection.

1.3 PERTINENT DATA

a. Drainage Area - 134 acres (includes 75 acres above two upstream reservoirs)

b. Discharge at Damsite.

(1) Normal discharge at the damsite is through an uncontrolled 36-inch corrugated metal pipe.

- (2) Estimated experienced maximum flood at damsite - Unknown.
- (3) Estimated ungated spillway capacity at maximum pool elevation - elevation-80 cfs (Probable Maximum Flood Pool El. 717.4)
 - c. Elevation (Feet above m.s.l. Approximate Tie to USGS Map).
 - (1) Top of dam - 714.4 (see Plate 3)
 - (2) Spillway outlet invert - 707.0
 - (3) Streambed at toe of dam - 670.5
 - (4) Maximum tailwater - Unknown.
 - d. Reservoir.
 - (1) Length of maximum pool - 1,800 feet \pm (Probable maximum flood pool level)
 - (2) Length of normal pool - 1,600 feet \pm (Spillway outlet invert)
 - e. Storage (Acre-feet).
 - (1) Top of dam - 200
 - (2) Spillway outlet invert - 118
 - (3) Design surcharge - Not available.
 - f. Reservoir Surface (Acres).
 - (1) Top of dam - 12.8
 - (2) Spillway outlet invert - 9.7
 - g. Dam.
 - (1) Type - Earth embankment.
 - (2) Length - 380 feet
 - (3) Height - 44 feet \pm
 - (4) Top width - 40 feet
 - (5) Side slopes - upstream face 1.0 V on 2.6 H, downstream face between 1.0 V on 3.4 H and 1.0 V on 4.2 H (see Plate 4).

- (6) Zoning - Unknown.
- (7) Impervious core - Unknown.
- (8) Cutoff - Unknown.
- (9) Grout curtain - Unknown.
- h. Diversion and Regulating Tunnel - None.
- i. Spillway.
 - (1) Type - 36-inch corrugated metal pipe.
 - (2) Inlet invert elevation - 705.0 feet m.s.l.
 - (3) Outlet invert elevation - 707.0 feet m.s.l.
 - (4) Gates - None.
 - (5) Upstream channel - The normal pool would back up to near the toe of an upstream dam.
 - (6) Downstream channel - Discharges to a ditch and then to the hillside.
- j. Emergency Spillway - None.
- k. Regulating Outlets - None.

SECTION 2 - ENGINEERING DATA

2.1 DESIGN

Design data were not available.

2.2 CONSTRUCTION

Construction records were unavailable; however, the owner's representative stated that the dam was designed by Mr. Dave Krehbeil and was constructed in the summer of 1979.

2.3 OPERATION

Operational records and documentation of past floods were unavailable.

2.4 GEOLOGY

The site of the dam and reservoir is located in a narrow, steep-sided valley in hilly terrain. The dam impounds a small intermittent side tributary of the Osage River which is dammed to form the Lake of the Ozarks.

The soils in the area of the dam and reservoir consist of the Lebanon, Doniphan, Gepp, Bardley and Clarksville soil series. The Lebanon soils are formed in loess overlying residuum weathered from cherty limestone or dolomite on ridgetops and upper side slopes. For engineering purposes, the soils are classified as CL material. The Doniphan soils are formed in residuum weathered from clayey shales and cherty dolomite on ridgetops and side slopes. For engineering purposes, the soils are classified as CL, CH, MH, GM, or SM-SC materials. The Gepp, Bardley and Clarksville soils are developed in residuum weathered from cherty dolomite. For engineering purposes, the soils are classified as GC, GM, SC, SM, ML, CL or CH materials depending on location of the samples.

The bedrock in the area of the dam and reservoir consists of dolomite with abundant chert of the Gasconade formation of the Canadian Series of the Ordovician System. The Gasconade formation forms nearly vertical bluffs and cliffs along streams in the central Ozarks and caves and springs are common.

2.5 EVALUATION

- a. Availability. No engineering data were available.
- b. Adequacy. No engineering data were available. Thus, an assessment of the design, construction, and operation could not be made.

Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency. These seepage and stability analyses should be performed for appropriate loading conditions (including earthquake loads) and made a matter of record.

c. Validity. The validity of the design, construction, and operation could not be determined due to the lack of engineering data.

SECTION 3 - VISUAL INSPECTION

3.1 FINDINGS

a. General. A visual inspection of Burton-Duenke Dam #4 was made on 24 April 1981. The inspection team consisted of Edwin Burton, team leader; Robert Pinker, geologist; Gary Van Riessen, geotechnical engineer; and John Ruhl, hydrologic/hydraulic engineer. Mr. Westhoff and Mr. Krehbiel, representatives of the owner, met the inspection team at the dam and provided information regarding design, construction, and maintenance. The dam appears to be in good condition. Specific observations are discussed below. No observations were made of the condition of the upstream face of the dam below the pool elevation at the time of the inspection.

b. Dam. The inspection team observed the following conditions at the dam. No cracking, sliding, sloughing, sinkholes, or other signs of settlement or instability were observed. The embankment has a broad crest with a flat downstream slope and a mild upstream slope. No toe drains, relief wells, or instruments to measure the performance of the dam were located.

Minor seepage was occurring at the interface of the left abutment and the downstream slope, below the inlet of the spillway pipe, and on both abutments at the upstream embankment slope. The area at the interface of the left abutment and the downstream slope was flowing clear at a rate of approximately 5 gallons/hour. Seepage below the spillway pipe and at the upstream/abutment interfaces was less than 5 gallons/hour, was above the lake level and was coming from the abutments. The area downstream of the dam was fairly wet, but because of rain on the day previous to the inspection, it was impossible to determine if this was due to seepage.

Minor erosion of silty clay material has taken place on the upstream and downstream slopes, at the embankment/abutment interfaces at each of the corners of the embankment, and below the inlet of the spillway pipe. There is no evidence to indicate that the embankment has ever been overtopped.

The ground cover on the embankment consists of thin weeds and no grass in rocky soil. There was no riprap on the embankment. There were no animal burrows or trees.

c. Appurtenant Structures. The spillway is the only appurtenant structure observed by the inspection team. The alignment of the spillway pipe was observed to dogleg to the left when viewed from the upstream end. There is a concrete headwall (8 feet wide, 5.5 feet high and 0.6 feet thick) at the pipe inlet. There was no trash rack at the pipe inlet.

There was approximately 2 to 3 inches of sediment in the pipe at the upstream end. About 6 feet and 4 feet of the outside of the pipe was observed at the upstream and downstream ends, respectively. One joint in the pipe was observed and was in good condition with no apparent movement. The survey made during the inspection showed the spillway pipe outlet to be about 2 feet higher than its inlet which appeared to be the way it was constructed. There is no evidence of leakage into, out of, or around the pipe. The pipe appears to be in good condition with no rust observed. The pipe discharges into a ditch which is approximately 3-1/2 feet wide and 2 feet deep. The ditch ends about 75 feet below the pipe outlet where discharge is released to the hillside north of the left abutment. There is no erosion downstream of the pipe outlet.

d. Geology. The soils in the area of the dam and reservoir consist of silty clay with numerous rock fragments ranging in diameter from 1/4 inch to 6 inches. The soil developed from residuum weathered from the underlying dolomite and chert bedrock. The soil is typically less than 5 feet thick.

The bedrock in the area of the dam and reservoir consists of dolomite with abundant chert and a 6 to 10-foot thick sandstone bed. The rocks are classified as the Gasconade formation; the sandstone is the upper portion of the Gunter member of the Gasconade formation.

The upper two-thirds of the abutments at the ends of the embankment consist of dolomite with abundant chert. The lower one-third consists of sandstone. Seepage was observed coming from the sandstone unit where it is exposed on the abutments of the upstream slope at approximately 10 feet above the water in the reservoir. The dolomite is vuggy and contains numerous chert nodules and beds.

Samples of the near-surface materials in the embankments were taken near the center of the downstream crest using an Oakfield sampler.

These materials were classified as silty clay with numerous chert fragments. For engineering purposes, these samples were classified as CL materials. Based on these samples, it is anticipated that the remainder of the embankment is constructed of similar silty clay (CL) material.

e. Reservoir Area. Minor slumping or sliding of the reservoir right and left banks was observed (Photos 14 and 15). This is probably due to excavation of borrow material from the reservoir and should have no detrimental effects on the dam. The lake was noted to be clear with no noticeable siltation and a visibility of 1-1/2 to 2 feet.

f. Downstream Channel. The spillway discharges to a ditch and then to the hillside north of the left abutment.

3.2 EVALUATION

The various deficiencies observed at the time of the inspection are not believed to represent an immediate safety hazard. They do, however, warrant monitoring and control.

The seepage should be monitored regularly for quality and quantity. Similar areas of seepage were observed in natural hillsides in the area with no adverse effects. Seepage can cause internal erosion creating cavities and underground channels, thereby weakening the embankment. The erosion gullies should be backfilled with suitable material and compacted. The embankment should be seeded to prevent erosion.

The doglegged alignment and adverse slope of the spillway pipe make it susceptible to clogging from debris hangup inside the pipe and to sediment in the pipe. Realignment of the spillway pipe or constructing an inlet baffle would reduce the potential for clogging.

The lack of good ground cover on the embankment has resulted in minor erosion of the embankment due to seepage and local runoff. The absence of riprap on the upstream face does not appear to be a problem.

SECTION 4 - OPERATIONAL PROCEDURES

4.1 PROCEDURES

The pool is primarily controlled by rainfall, runoff, evaporation, transpiration, seepage, and capacity of the uncontrolled spillway. The lake was considerably below normal pool at the time of the inspection.

4.2 MAINTENANCE OF DAM

There was no evidence of a regular maintenance program. The crest appeared to have been graded within a year prior to the inspection.

4.3 MAINTENANCE OF OPERATING FACILITIES

No operating facilities exist.

4.4 DESCRIPTION OF ANY WARNING SYSTEM IN EFFECT

There is no existing warning system or preplanned scheme for alerting downstream residents for this dam.

4.5 EVALUATION

A maintenance program should be established to include seeding the embankment with grass, mowing the grass and weed cover on the embankment when it is developed, and removal of any trees that appear.

SECTION 5 - HYDRAULIC/HYDROLOGIC

5.1 EVALUATION OF FEATURES

a. Design Data. No design data were available.

b. Experience Data. The drainage area and lake surface area are developed from the USGS Lake Ozark Quadrangle Map. The dam layout is from a survey made during the inspection.

c. Visual Observations.

(1) The spillway appears to be in good condition. The lake level at the time of the inspection (El. 698.0) was below the spillway outlet invert. There were no obstructions to flow in the downstream channel. There was approximately 2 to 3 inches of sediment in the pipe at the upstream end.

(2) There is no emergency spillway for this dam.

(3) Spillway discharges do not endanger the integrity of the dam.

d. Overtopping Potential. The spillway will not pass the probable maximum flood without overtopping the dam. The probable maximum flood is defined as the flood discharge that may be expected from the most severe combination of critical meteorologic and hydrologic conditions that are reasonably possible in the region. The spillway will pass 20 percent of the probable maximum flood without overtopping the dam. The spillway will pass the one percent chance flood estimated to have a peak outflow of 15 cfs developed by a 24-hour, one percent chance rainfall. According to the recommended guidelines from the Department of the Army, Office of the Chief of Engineers, a high hazard dam of intermediate size should pass the probable maximum flood. The portion of the estimated peak discharge of 50 percent of the probable maximum flood overtopping the dam would be 2,180 cfs of the total discharge from the reservoir of 2,260 cfs. The estimated duration of overtopping is 5.8 hours with a maximum height of 2.8 feet. The portion of the estimated peak discharge of the probable maximum flood overtopping the dam would be 2,540 cfs of the total discharge from the reservoir of 2,620 cfs. The estimated duration of overtopping is 8.8 hours with a maximum height of 3.0 feet. The embankment could be jeopardized should overtopping occur for these periods of time.

According to the St. Louis District, Corps of Engineers, the effect from rupture of the dam could extend approximately one mile downstream to the Lake of the Ozarks. A marina, two dwellings, and thirteen trailers could be severely damaged and lives could be lost should failure of the dam occur. Contents of the estimated downstream damage zone were verified

by the inspection team. There does not appear to be any flood plain regulations or other constraints in force to limit future downstream development.

SECTION 6 - STRUCTURAL STABILITY

6.1 EVALUATION OF STRUCTURAL STABILITY

a. Visual Observations. Visual observations of conditions which affect the structural stability of this dam are discussed in Section 3, paragraph 3.1b.

b. Design and Construction Data. No design data relating to the structural stability of the dam were found. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency.

c. Operating Records. No operational records exist.

d. Postconstruction Changes. The only postconstruction change which was observed is the grading on the crest. It appeared that this repair had been made within the past year.

e. Seismic Stability. The dam is located in Seismic Zone 1 which is a zone of minor seismic risk. A properly designed and constructed earth dam using sound engineering principles and conservatism should pose no serious stability problems during earthquakes in this zone. The seismic stability of an earth dam is dependent upon a number of factors: embankment and foundation material classifications and shear strengths; abutment materials, conditions, and strengths; embankment zoning; and embankment geometry. Adequate descriptions of embankment design parameters, foundation and abutment conditions, or static stability analyses to assess the seismic stability of this embankment were not available and therefore no inferences will be made regarding the seismic stability. An assessment of the seismic stability should be included as part of the stability analysis required by the guidelines.

SECTION 7 - ASSESSMENT/REMEDIAL MEASURES

7.1 DAM ASSESSMENT

a. Safety. Several conditions observed during the visual inspection by the inspection team should be monitored and/or controlled. There is seepage at the interface of the left abutment and the downstream slope, below the inlet of the spillway pipe, and on both abutments at the upstream embankment slope, erosion on the upstream and downstream slopes, at the embankment/abutment interfaces at each of the corners of the embankment, and below the inlet of the spillway pipe, and a very thin vegetal cover. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency.

b. Adequacy of Information. Due to the absence of engineering design data, the conclusions in this report were based only on performance history and visual conditions. The inspection team considers that these data are sufficient to support the conclusions herein. Seepage and stability analyses comparable to the requirements of the "Recommended Guidelines for Safety Inspection of Dams" were not available, which is considered a deficiency.

c. Urgency. It is the opinion of the inspection team that a program should be developed as soon as possible to implement remedial measures recommended in paragraph 7.2b. If the safety deficiencies listed in paragraph 7.1a are not corrected, they will continue to deteriorate and lead to a serious potential of failure. The item recommended in paragraph 7.2a should be pursued on a high priority basis.

d. Necessity for Phase II. The Phase I investigation does not raise any serious questions relating to the safety of the dam nor does it identify any serious dangers which would require a Phase II investigation. However, the additional analyses noted in paragraph 2.5b are necessary for compliance with the guidelines.

e. Seismic Stability. This dam is located in Seismic Zone 1. Adequate description of embankment design parameters, foundation and abutment conditions, or static stability analyses to assess the seismic stability of this embankment were not available and therefore no inferences will be made regarding the seismic stability. An assessment of the seismic stability should be included as part of the recommended stability analysis.

7.2 REMEDIAL MEASURES

a. Alternatives. The spillway capacity and/or storage volume would need to be increased or the lake level would need to be permanently lowered to increase available flood storage in order to effectively pass

the recommended spillway design flood. Spillway capacity could be increased by providing an emergency spillway. The storage volume could be increased by raising the low areas of the dam crest.

b. Operation and Maintenance Procedures. The following operation and maintenance procedures are recommended and should be carried out under the direction of a professional engineer experienced in the design, construction, and maintenance of earth dams.

(1) The seepage areas noted during the visual inspection should be closely monitored and documented as to quantity of flow. Any significant changes should be evaluated.

(2) The erosion gullies should be backfilled with suitable material and compacted.

(3) A debris baffle or a trash rack should be constructed at the spillway pipe inlet or the pipe should be realigned to a straight alignment to prevent clogging.

(4) A maintenance program to control the future growth of trees on the embankment should be developed. The embankment should be seeded with grass and developed for erosion protection. Grass/weed cover on the embankment should be cut periodically after it has been developed.

(5) Seepage and stability analyses should be performed.

(6) A detailed inspection of the dam should be made periodically. The findings of this inspection should be documented and made a matter of record. More frequent inspections may be required if additional deficiencies are observed or the severity of the reported deficiencies increase.

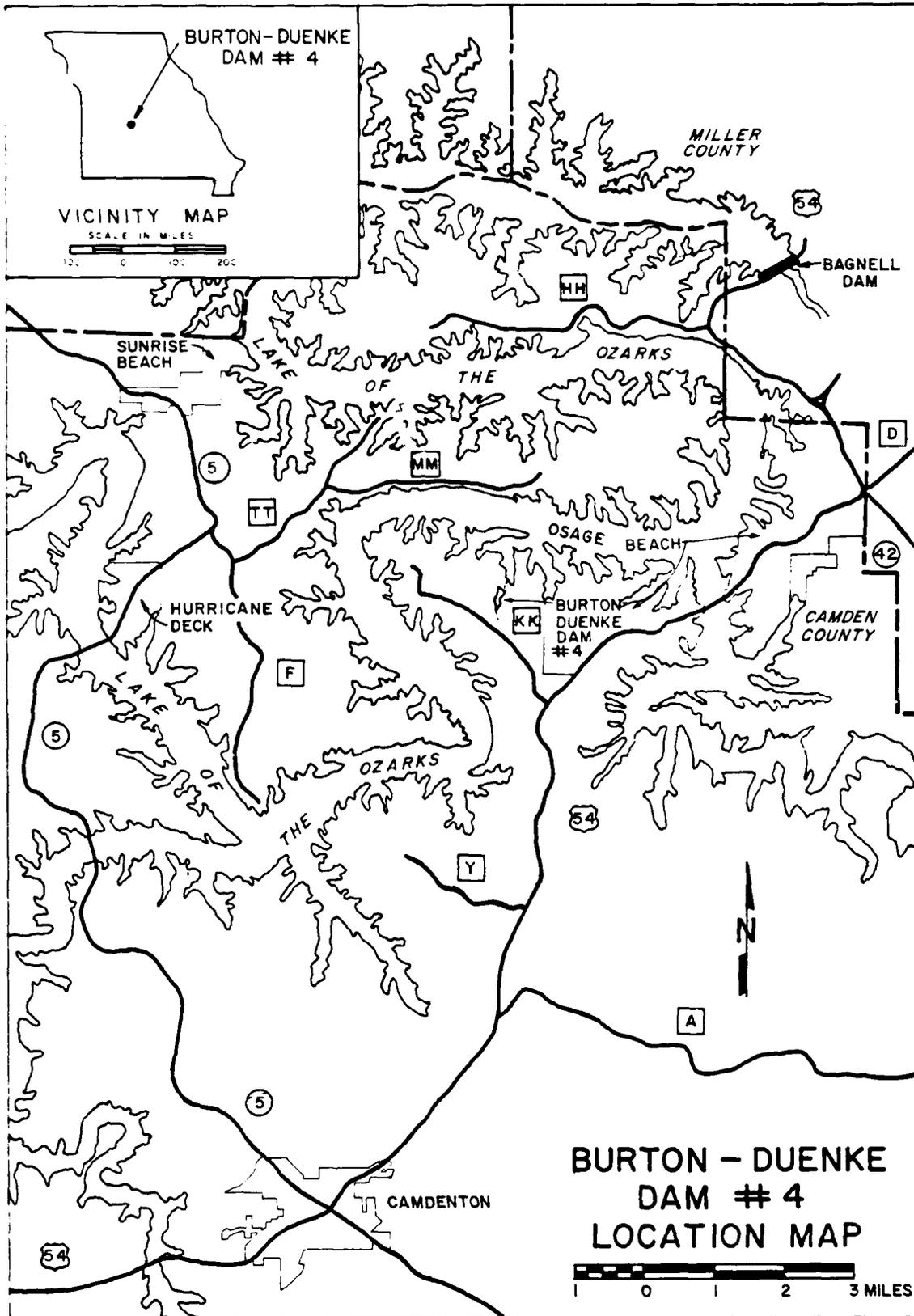
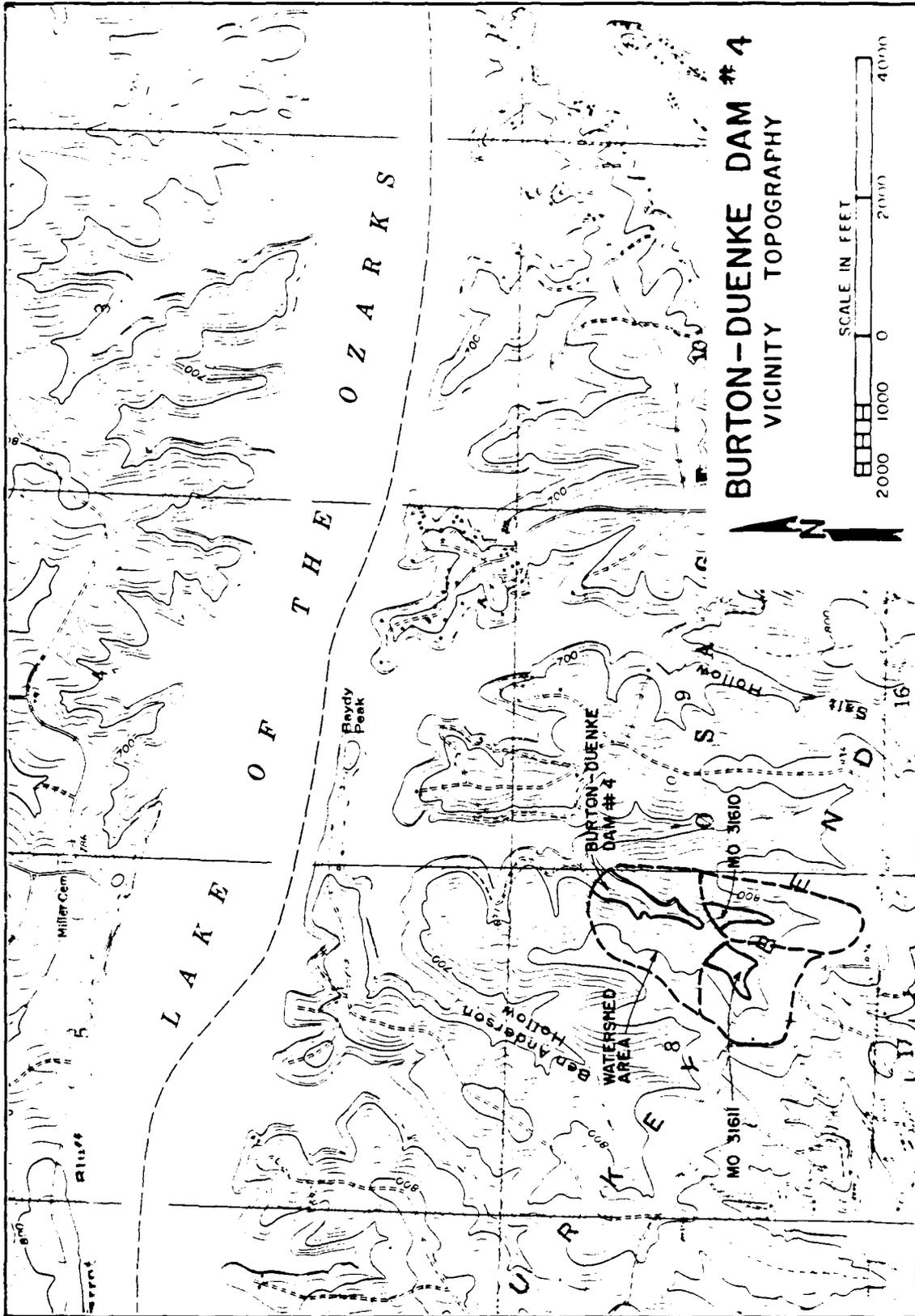


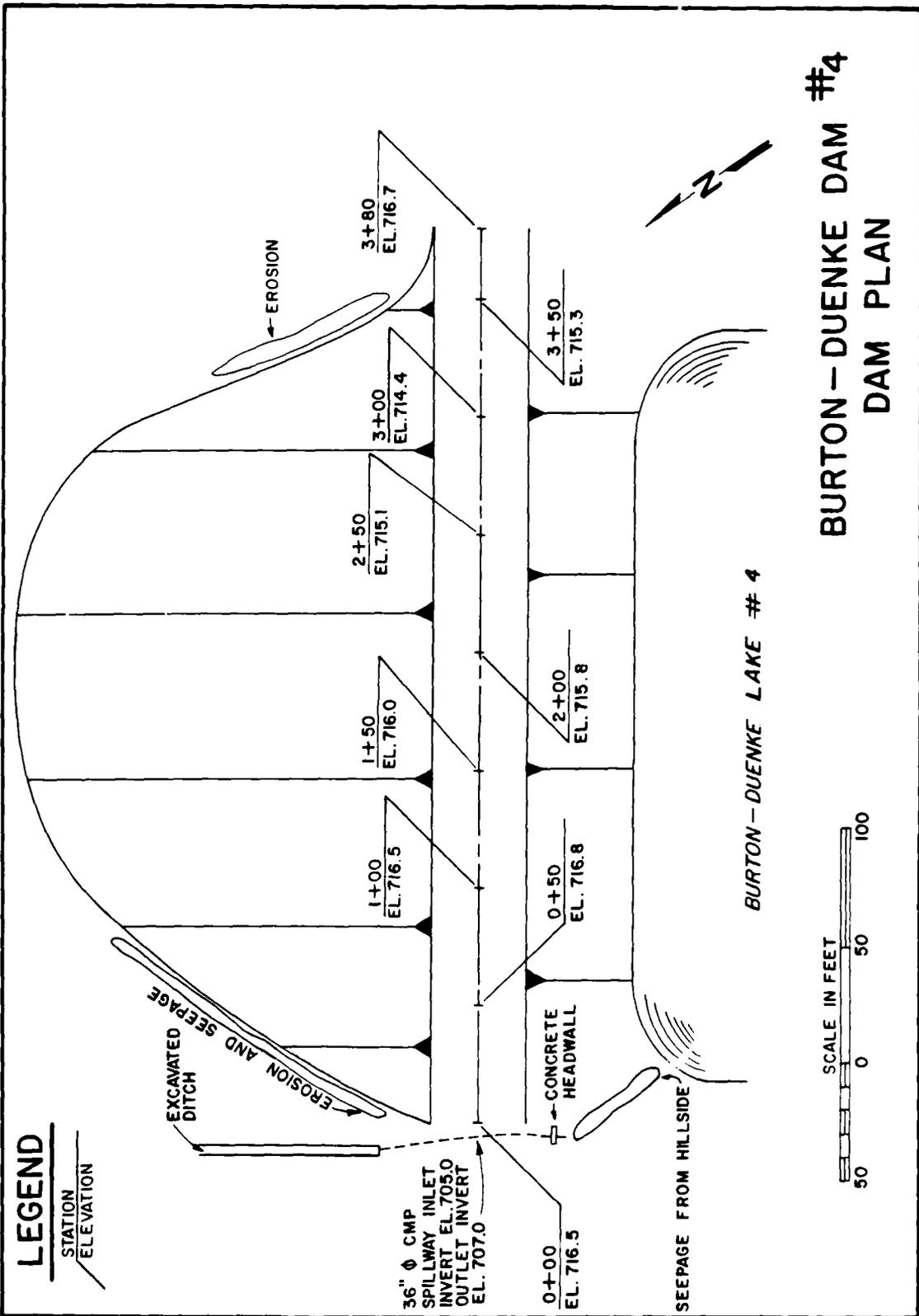
PLATE I



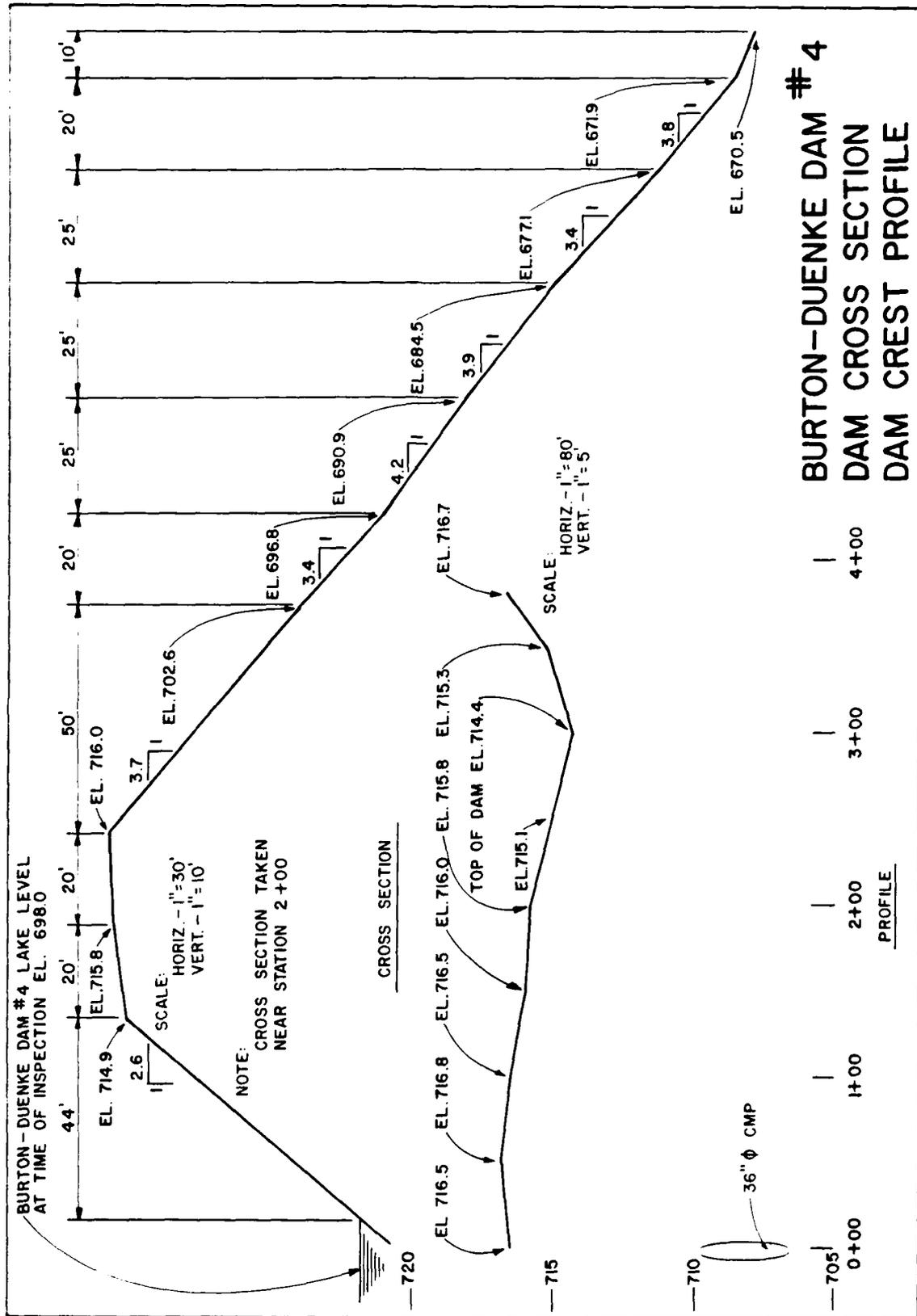
BURTON-DUENKE DAM #4
VICINITY TOPOGRAPHY

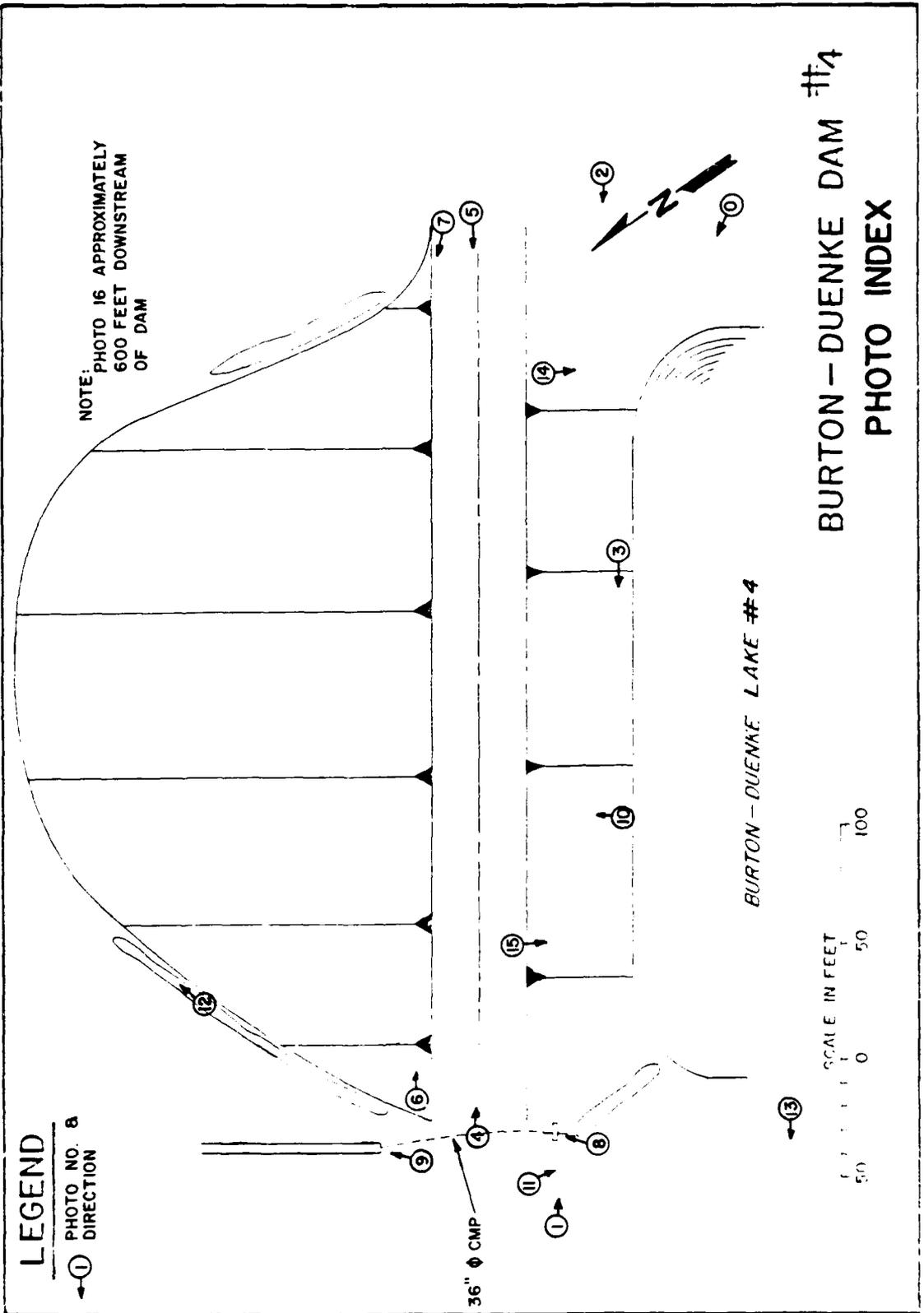


PLATE 2



**BURTON - DUENKE DAM #4
 DAM PLAN**





LEGEND

① PHOTO NO. & DIRECTION

NOTE: PHOTO 16 APPROXIMATELY 600 FEET DOWNSTREAM OF DAM

**BURTON - DUENKE DAM #4
PHOTO INDEX**

SCALE IN FEET
0 50 100



PHOTO 1 : UPSTREAM FACE OF DAM LOOKING EAST



PHOTO 2 : UPSTREAM FACE OF DAM LOOKING WEST



PHOTO 3 : UPSTREAM FACE OF DAM AT WATERLINE



PHOTO 4 : CREST OF DAM LOOKING EAST



PHOTO 7 - CREST OF DAM LOOKING WEST



PHOTO 8 - DOWNSTREAM FACE OF DAM LOOKING EAST



PHOTO 7 : DOWNSTREAM FACE OF DAM LOOKING WEST

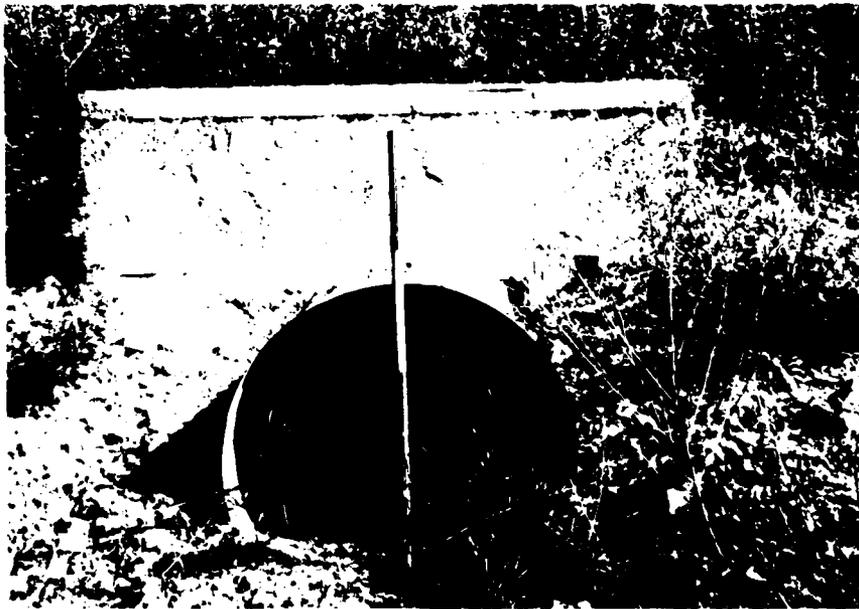


PHOTO 8 : SPILLWAY PIPE INLET



PHOTO 9 : CHANNEL DOWNSTREAM OF SPILLWAY PIPE OUTLET



PHOTO 10: EROSION ON UPSTREAM FACE OF DAM

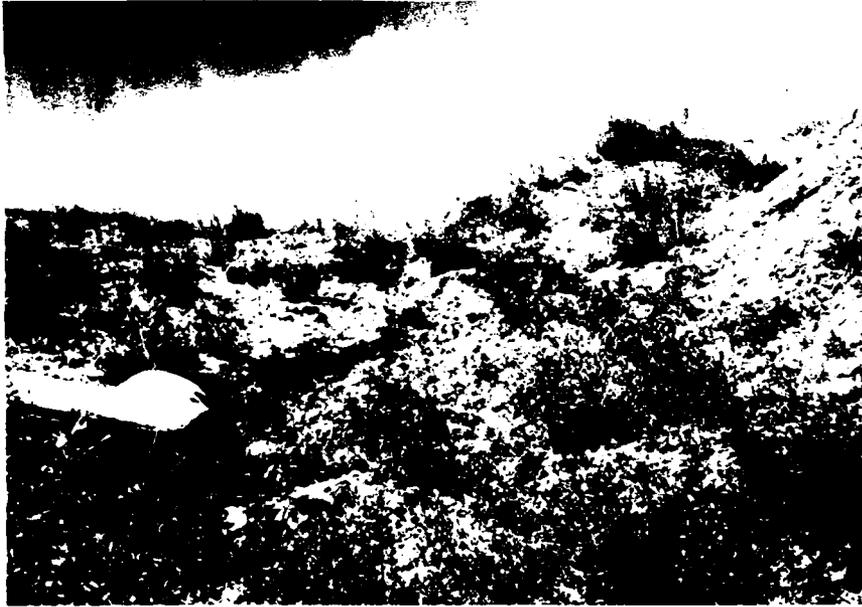


PHOTO 11: EROSION AT UPSTREAM FACE AND LEFT ABUTMENT



PHOTO 12: EROSION AT DOWNSTREAM FACE AND LEFT ABUTMENT



PHOTO 13: LEFT ABUTMENT JUST UPSTREAM OF DAM



PHOTO 14: RIGHT BANK OF RESERVOIR



PHOTO 15: LAKE AND WATERSHED VIEWED FROM DAM



PHOTO 16: VALLEY DOWNSTREAM OF DAM

APPENDIX A
HYDROLOGIC AND HYDRAULIC ANALYSES

HYDROLOGIC AND HYDRAULIC ANALYSES

To determine the overtopping potential of Burton-Duenke Dam #4, flood routings were performed by applying the Probable Maximum Precipitation (PMP) to a synthetic unit hydrograph to develop inflow hydrographs for the reservoirs being studied and the upstream reservoirs. The inflow hydrographs were then routed through the reservoirs and spillways. The overtopping analysis was determined using the computer program HEC-1 (Dam Safety Version) (1).

The PMP was determined from regional charts prepared by the National Weather Service in "Hydrometeorological Report No. 33" (HMR-33) (2). Reduction factors were not applied. The rainfall distribution for the 24-hour PMP storm was determined according to the procedures outlined in HMR-33 and EM 1110-2-1411 (3). The Jefferson City, Missouri rainfall distribution (5 min. interval - 24 hours duration), as provided by the St. Louis District, Corp of Engineers, was used when the one percent chance probability flood was routed through the reservoirs and spillways.

The synthetic unit hydrograph for the watershed was developed by the computer program using the Soil Conservation Service (SCS) method (1 and 4). The parameters for the unit hydrographs are shown in Table 1. The time of concentration (T_c) for the reservoir being studied was determined using the Kirpich method and was verified by the SCS method (4 and 5).

The SCS curve number (CN) method was used in computing the infiltration losses for the rainfall-runoff relationship. The CN values used, and the result from the computer output, are shown in Table 2.

Two impoundments in the watershed of Burton-Duenke Dam #4 were included in the hydrologic and hydraulic analyses. Storms were routed through these lakes (see Plate 2) which shall be referenced as "Dam #1" and "Dam #2" through the remainder of this appendix. Input data for the analysis of each of these dams was assumed from previous reports on these structures (6 and 7).

Routing through the reservoirs was performed using the Modified Puls Method. The initial reservoir pool elevations for the routing of each storm were determined to be equivalent to the inlet or outlet invert elevations of the spillways in accordance with antecedent storm conditions preceding the one percent probability and probable maximum storms outlined by the U.S. Army Corps of Engineers, St. Louis District (8). The hydraulic capacity of the spillways and the storage capacity of the reservoir were defined by the elevation, surface area, storage, and discharge relationships shown in Table 3.

The flow over the crest of the dams was determined using the non-level dam crest option (\$L and \$V cards) of the HEC-1 program. The

program assumes critical flow over a broad-crested weir. The flow through the spillways was determined from Hydraulic Charts for the Selection of Highway Culverts (9).

Where routing through the upstream reservoirs resulted in overtopping of those structures, a breach analysis was performed using HEC-1. The breaching parameters are noted in Table 4.

The result of the routing analysis indicates that the spillway under study will pass a flood equivalent to 20 percent of the PMF without overtopping the dam.

A summary of the routing analysis for different ratios of the PMF is shown in Table 5.

The computer input data and a summary of the output data are presented at the back of this appendix.

TABLE 1
SYNTHETIC UNIT HYDROGRAPH

<u>Parameters:</u>	<u>Dam #4</u>	<u>Dam #1</u>	<u>Dam #2</u>
Drainage Area (A)	134 acres	40 acres	35 acres
Length of Longest Watercourse (L)	0.26 miles	0.25 miles	0.13 miles
Elevation Difference in Watershed (H)	118 feet	107 feet	81 feet
Lag Time (L_g)	0.05 hours	0.05 hours	0.03 hours
Time of concentration (T_c)	0.09 hours	0.09 hours	0.05 hours
Duration (D)	0.7 min. (use 5 minutes in each case)	0.7 min.	0.4 min.

<u>Time (Min.) *</u>	<u>Discharge (cfs) *</u>		
	<u>Dam #4</u>	<u>Dam #1</u>	<u>Dam #2</u>
0	0	0	0
5	476	326	316
10	182	125	89
15	42	29	17
20	10	7	3
25	2	1	0
30	0	0	0

* From HEC-1 computer output

FORMULAS USED:

$$T_c = (11.9 \times L^3/H) \cdot 385 \quad (5)$$

$$L_g = 0.6 T_c$$

$$D = 0.133 T_c$$

TABLE 2
RAINFALL-RUNOFF VALUES

<u>Selected Storm Event</u>	<u>Storm Duration (Hours)</u>	<u>Rainfall (Inches)</u>	<u>Runoff (Inches)</u>	<u>Loss (Inches)</u>
PMP				
Dam #4	24	33.41	30.75	2.66
Dam #1	24	33.41	30.56	2.85
Dam #2	24	33.41	30.63	2.78
50% PMP				
Dam #4	24	17.91	15.38	2.53
Dam #1	24	18.28	15.28	3.00
Dam #2	24	18.31	15.31	3.00
1% Probability				
Dam #4	24	7.44	3.34	4.10
Dam #1	24	7.44	3.37	4.07
Dam #2	24	7.44	3.46	3.98

Additional Data:

- 1) The soil associations in this watershed are Gebb, Bardley, Clarksville, Lebanon, and Doniphan (10).
 51 percent of total drainage area in hydrologic soil group B.
 49 percent of total drainage area in hydrologic soil group C.
 80 percent of the land use was timberland
 15 percent of the land use was grassland
 5 percent of the land use was urban (4 and 11)
- 2) SCS Runoff Curve CN (AMC III) for ratios of the PMF:
 81 - Dam #4
 78 - Dam #1
 78 - Dam #2
- 3) SCS Runoff Curve CN (AMC II) for the one percent probability flood:
 64 - Dam #4
 60 - Dam #1
 60 - Dam #2

TABLE 3

ELEVATION, SURFACE AREA, STORAGE, AND DISCHARGE RELATIONSHIPS

<u>Elevation (feet-MSL)</u>	<u>Lake Surface Area (acres)</u>	<u>Lake Storage (acre-ft)</u>	<u>Spillway Discharge (cfs)</u>
Dam #4			
*707.0	9.7	118	0
709.5	10.7	143	24
712.0	11.8	170	53
***714.4	12.8	200	66
Dam #1			
**748.4	3.7	42	0
750.0	3.9	48	10
***751.3	4.2	53	24
Dam #2			
**773.6	4.0	55	0
775.0	4.2	61	7
***777.1	4.5	70	20

*Spillway outlet invert elevation
 **Spillway inlet invert elevation
 ***Top of dam elevation

The relationships in Table 3 were developed from the Lake Ozark, Missouri 7.5 minute quadrangle map and the field measurements.

METHOD USED

Spillway releases were determined by nomographs for corrugated metal pipe culverts with inlet and outlet control (9).

TABLE 4

BREACHING PARAMETERS

	<u>Dam #1</u>	<u>Dam #2</u>
Bottom Width of Breach (BRWID)	10 feet	10 feet
Side Slope of Breach (Z) (In feet horizontal to 1.0 foot vertical)	0.5 feet	0.5 feet
Elevation of Breach Bottom at Maximum Size of Breach (ELBM)	724.7 ft. m.s.l.	735.4 ft. m.s.l.
Time for Breach to Develop to Maximum Size (TFAIL)	1.0 hour	1.0 hour
Elevation of Water Surface Which Will Cause Dam to Fail (FAILEL)	751.3 ft. m.s.l.	777.1 ft. m.s.l.

TABLE 5

RESULTS OF FLOOD ROUTINGS

Ratio of PMF	Peak Inflow (CFS)	Peak Lake Elevation (FT.-MSL)	Total Storage (AC.-FT.)	Peak Outflow (CFS)	Depth (FT.) Over Top of Dam	Duration (HR.) Of Overtopping
-	0	*707.0	118	0	-	-
0.20	1,120	714.3	198	65	0	0
0.50	2,410	717.2	237	2,260	2.8	5.8
1.00	3,200	717.4	239	2,620	3.0	8.8

* Spillway outlet invert elevation

BIBLIOGRAPHY

- (1) U.S. Army Corps of Engineers, Hydrologic Engineering Center, Flood Hydrograph Package (HEC-1), Dam Safety Version, July 1978, Davis, California.
- (2) HMR-33, Seasonal Variations of Probable Maximum Precipitation, East of the 105th Meridian for Areas 10 to 1,000 Square Miles and Durations from 6 to 48 Hours, U.S. Department of Commerce, NOAA, National Weather Service, 1956.
- (3) EM-1110-2-1411, Standard Project Flood Determinations, U.S. Army Corps of Engineers, 26 March 1952.
- (4) U.S. Department of Agriculture, Soil Conservation Service, National Engineering Handbook, Section 4, Hydrology, August 1972.
- (5) U.S. Department of the Interior, Bureau of Reclamation, Design of Small Dams, 1974, Washington, D.C.
- (6) U.S. Army Corps of Engineers, St. Louis District, Burton Duenke #1 Lake Dam, Phase I Inspection Report, August 1980.
- (7) U.S. Army Corps of Engineers, St. Louis District, Burton Duenke #2 Lake Dam, Phase I Inspection Report, July 1980.
- (8) U.S. Army Corps of Engineers, St. Louis District, Hydrologic/Hydraulic Standards, Phase I Safety Inspection of Non-Federal Dams, 12 December 1979.
- (9) U.S. Department of Commerce, Bureau of Public Roads, Hydraulic Engineering Circular No. 5, Hydraulic Charts for the Selection of Highway Culverts, December 1965.
- (10) U.S. Department of Agriculture, Soil Conservation Service, Preliminary Soils Report for Camden County, Missouri.
- (11) U.S. Department of Agriculture, Soil Conservation Service, Technical Release No. 55, Urban Hydrology for Small Watersheds, January 1975.
- (12) Mary H. McCracken, Missouri Division of Geological Survey, Geologic Map of Missouri, 1961.
- (13) John W. Koenig, Missouri Division of Geological Survey, The Stratigraphic Succession in Missouri, 1961.

MO. DA	HR. PM	PERIOD	RAIN	EXCS	LOSS	END-OF-PERIOD FLOW	MO. DA	HR. PM	PERIOD	RAIN	EXCS	LOSS	COMP. #
1-01	05	1	.01	.00	.01	1-01	12:05	145	.22	.19	.02	72.	
1-01	10	2	.01	.00	.01	1-01	12:10	146	.22	.20	.02	90.	
1-01	15	3	.01	.00	.01	1-01	12:15	147	.22	.20	.02	95.	
1-01	20	4	.01	.00	.01	1-01	12:20	148	.22	.20	.02	96.	
1-01	25	5	.01	.00	.01	1-01	12:25	149	.22	.20	.02	97.	
1-01	30	6	.01	.00	.01	1-01	12:30	150	.22	.20	.02	97.	
1-01	35	7	.01	.00	.01	1-01	12:35	151	.22	.20	.02	98.	
1-01	40	8	.01	.00	.01	1-01	12:40	152	.22	.20	.02	98.	
1-01	45	9	.01	.00	.01	1-01	12:45	153	.22	.20	.02	98.	
1-01	50	10	.01	.00	.01	1-01	12:50	154	.22	.20	.02	99.	
1-01	55	11	.01	.00	.01	1-01	12:55	155	.22	.20	.02	99.	
1-01	00	12	.01	.00	.01	1-01	13:00	156	.22	.20	.02	99.	
1-01	05	13	.01	.00	.01	1-01	13:05	157	.26	.25	.02	113.	
1-01	10	14	.01	.00	.01	1-01	13:10	158	.26	.25	.02	118.	
1-01	15	15	.01	.00	.01	1-01	13:15	159	.26	.25	.01	120.	
1-01	20	16	.01	.00	.01	1-01	13:20	160	.26	.25	.01	121.	
1-01	25	17	.01	.00	.01	1-01	13:25	161	.26	.25	.01	121.	
1-01	30	18	.01	.00	.01	1-01	13:30	162	.26	.25	.01	121.	
1-01	35	19	.01	.00	.01	1-01	13:35	163	.26	.25	.01	122.	
1-01	40	20	.01	.00	.01	1-01	13:40	164	.26	.25	.01	122.	
1-01	45	21	.01	.00	.01	1-01	13:45	165	.26	.25	.01	122.	
1-01	50	22	.01	.00	.01	1-01	13:50	166	.26	.25	.01	122.	
1-01	55	23	.01	.00	.01	1-01	13:55	167	.26	.25	.01	123.	
1-01	00	24	.01	.00	.01	1-01	14:00	168	.26	.25	.01	123.	
1-01	05	25	.01	.00	.01	1-01	14:05	169	.33	.32	.01	144.	
1-01	10	26	.01	.00	.01	1-01	14:10	170	.33	.32	.01	152.	
1-01	15	27	.01	.00	.01	1-01	14:15	171	.33	.32	.01	154.	
1-01	20	28	.01	.00	.01	1-01	14:20	172	.33	.32	.01	154.	
1-01	25	29	.01	.00	.01	1-01	14:25	173	.33	.32	.01	155.	
1-01	30	30	.01	.00	.01	1-01	14:30	174	.33	.32	.01	155.	
1-01	35	31	.01	.00	.01	1-01	14:35	175	.33	.32	.01	155.	
1-01	40	32	.01	.00	.01	1-01	14:40	176	.33	.32	.01	155.	
1-01	45	33	.01	.00	.01	1-01	14:45	177	.33	.32	.01	155.	
1-01	50	34	.01	.00	.01	1-01	14:50	178	.33	.32	.01	156.	
1-01	55	35	.01	.00	.01	1-01	14:55	179	.33	.32	.01	156.	
1-01	00	36	.01	.00	.01	1-01	15:00	180	.33	.32	.01	156.	
1-01	05	37	.01	.00	.01	1-01	15:05	181	.20	.19	.00	115.	
1-01	10	38	.01	.00	.01	1-01	15:10	182	.40	.39	.01	143.	
1-01	15	39	.01	.00	.01	1-01	15:15	183	.40	.39	.01	144.	
1-01	20	40	.01	.00	.01	1-01	15:20	184	.60	.59	.01	253.	
1-01	25	41	.01	.00	.01	1-01	15:25	185	.70	.68	.01	310.	
1-01	30	42	.01	.00	.01	1-01	15:30	186	1.69	1.67	.03	648.	
1-01	35	43	.01	.00	.01	1-01	15:35	187	2.79	2.75	.04	1129.	
1-01	40	44	.01	.00	.01	1-01	15:40	188	1.10	1.08	.01	750.	
1-01	45	45	.01	.00	.01	1-01	15:45	189	.70	.69	.01	451.	
1-01	50	46	.01	.00	.01	1-01	15:50	190	.60	.59	.01	331.	
1-01	55	47	.01	.00	.01	1-01	15:55	191	.40	.39	.00	234.	
1-01	00	48	.01	.00	.01	1-01	16:00	192	.40	.39	.00	201.	
1-01	05	49	.01	.00	.01	1-01	16:05	193	.31	.30	.00	164.	
1-01	10	50	.01	.00	.01	1-01	16:10	194	.31	.30	.00	151.	
1-01	15	51	.01	.00	.01	1-01	16:15	195	.31	.30	.00	149.	
1-01	20	52	.01	.00	.01	1-01	16:20	196	.31	.30	.00	148.	
1-01	25	53	.01	.00	.01	1-01	16:25	197	.31	.30	.00	148.	

1.01	4.30	54	.01	.00	.01	1.01	16.30	198	.31	.30	.00	148.
1.01	4.35	55	.01	.00	.01	1.01	16.35	199	.31	.30	.00	148.
1.01	4.40	56	.01	.00	.01	1.01	16.40	200	.31	.30	.00	148.
1.01	4.45	57	.01	.00	.01	1.01	16.45	201	.31	.30	.00	148.
1.01	4.50	58	.01	.00	.01	1.01	16.50	202	.31	.30	.00	148.
1.01	4.55	59	.01	.00	.01	1.01	16.55	203	.31	.30	.00	148.
1.01	5.00	60	.01	.00	.01	1.01	17.00	204	.31	.30	.00	148.
1.01	5.05	61	.01	.00	.01	1.01	17.05	205	.24	.24	.00	127.
1.01	5.10	62	.01	.00	.01	1.01	17.10	206	.24	.24	.00	119.
1.01	5.15	63	.01	.00	.01	1.01	17.15	207	.24	.24	.00	117.
1.01	5.20	64	.01	.00	.01	1.01	17.20	208	.24	.24	.00	116.
1.01	5.25	65	.01	.00	.01	1.01	17.25	209	.24	.24	.00	116.
1.01	5.30	66	.01	.00	.01	1.01	17.30	210	.24	.24	.00	116.
1.01	5.35	67	.01	.00	.01	1.01	17.35	211	.24	.24	.00	116.
1.01	5.40	68	.01	.00	.01	1.01	17.40	212	.24	.24	.00	116.
1.01	5.45	69	.01	.00	.01	1.01	17.45	213	.24	.24	.00	116.
1.01	5.50	70	.01	.00	.01	1.01	17.50	214	.24	.24	.00	116.
1.01	5.55	71	.01	.00	.01	1.01	17.55	215	.24	.24	.00	116.
1.01	6.00	72	.01	.00	.01	1.01	18.00	216	.24	.24	.00	116.
1.01	6.05	73	.06	.02	.04	1.01	18.05	217	.02	.02	.00	106.
1.01	6.10	74	.06	.02	.04	1.01	18.10	218	.02	.02	.00	99.
1.01	6.15	75	.06	.03	.04	1.01	18.15	219	.02	.02	.00	92.
1.01	6.20	76	.06	.03	.04	1.01	18.20	220	.02	.02	.00	86.
1.01	6.25	77	.06	.03	.04	1.01	18.25	221	.02	.02	.00	80.
1.01	6.30	78	.06	.03	.03	1.01	18.30	222	.02	.02	.00	75.
1.01	6.35	79	.06	.03	.03	1.01	18.35	223	.02	.02	.00	70.
1.01	6.40	80	.06	.03	.03	1.01	18.40	224	.02	.02	.00	65.
1.01	6.45	81	.06	.03	.03	1.01	18.45	225	.02	.02	.00	61.
1.01	6.50	82	.06	.03	.03	1.01	18.50	226	.02	.02	.00	57.
1.01	6.55	83	.06	.03	.03	1.01	18.55	227	.02	.02	.00	53.
1.01	7.00	84	.06	.04	.03	1.01	19.00	228	.02	.02	.00	49.
1.01	7.05	85	.06	.04	.03	1.01	19.05	229	.02	.02	.00	46.
1.01	7.10	86	.06	.04	.03	1.01	19.10	230	.02	.02	.00	43.
1.01	7.15	87	.06	.04	.03	1.01	19.15	231	.02	.02	.00	40.
1.01	7.20	88	.06	.04	.03	1.01	19.20	232	.02	.02	.00	37.
1.01	7.25	89	.06	.04	.02	1.01	19.25	233	.02	.02	.00	35.
1.01	7.30	90	.06	.04	.02	1.01	19.30	234	.02	.02	.00	33.
1.01	7.35	91	.06	.04	.02	1.01	19.35	235	.02	.02	.00	30.
1.01	7.40	92	.06	.04	.02	1.01	19.40	236	.02	.02	.00	28.
1.01	7.45	93	.06	.04	.02	1.01	19.45	237	.02	.02	.00	26.
1.01	7.50	94	.06	.04	.02	1.01	19.50	238	.02	.02	.00	25.
1.01	7.55	95	.06	.04	.02	1.01	20.00	239	.02	.02	.00	23.
1.01	8.00	96	.06	.04	.02	1.01	20.00	240	.02	.02	.00	21.
1.01	8.05	97	.06	.04	.02	1.01	20.05	241	.02	.02	.00	20.
1.01	8.10	98	.06	.04	.02	1.01	20.10	242	.02	.02	.00	19.
1.01	8.15	99	.06	.05	.02	1.01	20.15	243	.02	.02	.00	17.
1.01	8.20	100	.06	.05	.02	1.01	20.20	244	.02	.02	.00	16.
1.01	8.25	101	.06	.05	.02	1.01	20.25	245	.02	.02	.00	15.
1.01	8.30	102	.06	.05	.02	1.01	20.30	246	.02	.02	.00	14.
1.01	8.35	103	.06	.05	.02	1.01	20.35	247	.02	.02	.00	13.
1.01	8.40	104	.06	.05	.02	1.01	20.40	248	.02	.02	.00	12.
1.01	8.45	105	.06	.05	.02	1.01	20.45	249	.02	.02	.00	12.
1.01	8.50	106	.06	.05	.02	1.01	20.50	250	.02	.02	.00	11.
1.01	8.55	107	.06	.05	.02	1.01	20.55	251	.02	.02	.00	10.
1.01	9.00	108	.06	.05	.01	1.01	21.00	252	.02	.02	.00	10.
1.01	9.05	109	.06	.05	.01	1.01	21.05	253	.02	.02	.00	10.

TIME	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1-01 9-10	110	06	24	1-01 21-10	254
1-01 9-15	111	06	24	1-01 21-15	255
1-01 9-20	112	06	25	1-01 21-20	256
1-01 9-25	113	06	25	1-01 21-25	257
1-01 9-30	114	06	25	1-01 21-30	258
1-01 9-35	115	06	25	1-01 21-35	259
1-01 9-40	116	06	25	1-01 21-40	260
1-01 9-45	117	06	25	1-01 21-45	261
1-01 9-50	118	06	25	1-01 21-50	262
1-01 9-55	119	06	26	1-01 21-55	263
1-01 10-00	120	06	26	1-01 22-00	264
1-01 10-05	121	06	26	1-01 22-05	265
1-01 10-10	122	06	26	1-01 22-10	266
1-01 10-15	123	06	26	1-01 22-15	267
1-01 10-20	124	06	26	1-01 22-20	268
1-01 10-25	125	06	26	1-01 22-25	269
1-01 10-30	126	06	26	1-01 22-30	270
1-01 10-35	127	06	26	1-01 22-35	271
1-01 10-40	128	06	27	1-01 22-40	272
1-01 10-45	129	06	27	1-01 22-45	273
1-01 10-50	130	06	27	1-01 22-50	274
1-01 10-55	131	06	27	1-01 22-55	275
1-01 11-00	132	06	27	1-01 23-00	276
1-01 11-05	133	06	27	1-01 23-05	277
1-01 11-10	134	06	27	1-01 23-10	278
1-01 11-15	135	06	27	1-01 23-15	279
1-01 11-20	136	06	27	1-01 23-20	280
1-01 11-25	137	06	27	1-01 23-25	281
1-01 11-30	138	06	27	1-01 23-30	282
1-01 11-35	139	06	27	1-01 23-35	283
1-01 11-40	140	06	27	1-01 23-40	284
1-01 11-45	141	06	28	1-01 23-45	285
1-01 11-50	142	06	28	1-01 23-50	286
1-01 11-55	143	06	28	1-01 23-55	287
1-01 12-00	144	06	28	1-02 -00	288

SUM 33.41 30.56 2.85 15926.
 (849.3) (776.3) (72.3) (450.97)

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1129.	173.	55.	55.	15925.
32.	5.	2.	2.	451.
	25.52	32.66	32.66	32.66
	648.32	829.50	829.50	829.50
	26.	110.	110.	110.
	106.	135.	135.	135.

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 1

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
113.	17.	6.	6.	1592.
3.	0.	0.	0.	45.
	2.55	3.27	3.27	3.27
	64.83	82.95	82.95	82.95

CFS
 INCHES
 MM
 AC-FT
 THOUS CU M

AC-FT 9. 11. 11. 11.
 THOUS CU M 11. 16. 14. 14.

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 2

PEAK 6-HOUR 24-HOUR 72-HOUR TOTAL VOLUME
 226. 11. 3785.
 6. 1. 90.
 CFS 5.10 6.53 6.53
 INCHES 129.66 165.90 165.90
 MM 17. 22. 22.
 AC-FT 21. 27. 27.
 THOUS CU M

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 3

PEAK 6-HOUR 24-HOUR 72-HOUR TOTAL VOLUME
 282. 14. 3981.
 8. 1. 113.
 CFS 6.38 8.16 8.16
 INCHES 162.08 207.58 207.58
 MM 21. 27. 27.
 AC-FT 26. 34. 34.
 THOUS CU M

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 4

PEAK 6-HOUR 24-HOUR 72-HOUR TOTAL VOLUME
 339. 17. 4777.
 10. 1. 135.
 CFS 7.66 9.80 9.80
 INCHES 194.49 248.85 248.85
 MM 32. 33. 33.
 AC-FT 32. 41. 41.
 THOUS CU M

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 5

PEAK 6-HOUR 24-HOUR 72-HOUR TOTAL VOLUME
 395. 19. 5574.
 11. 1. 158.
 CFS 8.93 11.43 11.43
 INCHES 226.91 290.33 290.33
 MM 30. 38. 38.
 AC-FT 37. 47. 47.
 THOUS CU M

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 6

PEAK 6-HOUR 24-HOUR 72-HOUR TOTAL VOLUME
 453. 22. 6370.
 12. 1. 173.
 CFS 10.76 13.95 13.95
 INCHES 273.12 354.33 354.33
 MM 34. 43. 43.
 AC-FT 44. 55. 55.
 THOUS CU M

B L A 2 V E A T E M
 FLOOD HYDROGRAPH PACKAGE - HEC-1

CFS 451. 69. 22. 22. 6370.
 CMS 73. 1. 1. 1. 180.
 INCHES 10.21 13.06 13.06 13.06 13.06
 MM 259.33 331.80 331.80 331.80 331.80
 AC-FT 34. 44. 44. 44. 44.
 THOUS CU M 42. 54. 54. 54. 54.

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 7

PEAK 564. 86. 28. 28. 7962.
 CFS 16. 1. 1. 1. 225.
 CMS 12.76 16.33 16.33 16.33 16.33
 INCHES 324.16 414.75 414.75 414.75 414.75
 MM 43. 55. 55. 55. 55.
 AC-FT 53. 68. 68. 68. 68.
 THOUS CU M

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 8

PEAK 846. 130. 41. 41. 19943.
 CFS 24. 1. 1. 1. 538.
 CMS 19.14 24.49 24.49 24.49 24.49
 INCHES 486.24 622.13 622.13 622.13 622.13
 MM 64. 82. 82. 82. 82.
 AC-FT 79. 101. 101. 101. 101.
 THOUS CU M

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 9

PEAK 1129. 173. 55. 55. 15925.
 CFS 32. 2. 2. 2. 651.
 CMS 25.52 32.66 32.66 32.66 32.66
 INCHES 648.32 829.50 829.50 829.50 829.50
 MM 86. 110. 110. 110. 110.
 AC-FT 100. 135. 135. 135. 135.
 THOUS CU M

HYDROGRAPH ROUTING

ROUTE HYDROGRAPH THROUGH DAM #1

ISTAG 2 1 0 0 0 0
 ICOMP 1 0 0 0 0 0
 ISECON 0 0 0 0 0 0
 ISTAGE 0 0 0 0 0 0
 JPLT 0 0 0 0 0 0
 JPRT 4 1 1 1 1 1
 IMAINE 0 0 0 0 0 0
 IASTAG 0 0 0 0 0 0

STAGE	748.40	749.00	750.00	751.00	751.30	752.00	753.00	754.00	755.00
FLOW	.00	4.00	10.00	20.00	24.00	28.00	33.00	37.00	41.00
CAPACITY	0.	4.	18.	42.	48.	53.	53.	53.	53.
ELEVATION	722.	730.	740.	748.	750.	751.	751.	760.	760.

CREST LENGTH AT OR BELOW ELEVATION	0.	85.	180.	305.	330.	370.	415.	470.
TOPEL	751.3	751.4	751.6	751.7	752.1	753.2	754.0	755.0

DAM DATA

TOPEL	751.3	751.4	751.6	751.7	752.1	753.2	754.0	755.0
COOD	.0	.0	.0	.0	.0	.0	.0	.0
EXPD	.0	.0	.0	.0	.0	.0	.0	.0
DAMWID	.0	.0	.0	.0	.0	.0	.0	.0

DAM BREACH DATA

DAMID	10.	10.	10.	10.	10.	10.	10.	10.
Z	.50	.50	.50	.50	.50	.50	.50	.50
ELON	724.70	724.70	724.70	724.70	724.70	724.70	724.70	724.70
IFAIL	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
WSEL	748.40	748.40	748.40	748.40	748.40	748.40	748.40	748.40
FAILEL	751.30	751.30	751.30	751.30	751.30	751.30	751.30	751.30

PEAK OUTFLOW IS 11. AT TIME 18.08 HOURS

BEGIN DAM FAILURE AT 15.92 HOURS

PEAK OUTFLOW IS 1062. AT TIME 16.69 HOURS

BEGIN DAM FAILURE AT 15.67 HOURS

PEAK OUTFLOW IS 1083. AT TIME 16.44 HOURS

BEGIN DAM FAILURE AT 15.50 HOURS

PEAK OUTFLOW IS 1109. AT TIME 16.29 HOURS

BEGIN DAM FAILURE AT 15.33 HOURS

PEAK OUTFLOW IS 1157. AT TIME 16.04 HOURS

BEGIN DAM FAILURE AT 14.83 HOURS

PEAK OUTFLOW IS 1332. AT TIME 15.65 HOURS

BEGIN DAM FAILURE AT 14.17 HOURS
 PEAK OUTFLOW IS 1122. AT TIME 14.96 HOURS
 BEGIN DAM FAILURE AT 13.17 HOURS
 PEAK OUTFLOW IS 1138. AT TIME 13.96 HOURS
 BEGIN DAM FAILURE AT 12.58 HOURS
 PEAK OUTFLOW IS 1165. AT TIME 13.37 HOURS

***** SUB-AREA RUNOFF COMPUTATION *****

CALCULATE INFLOW HYDROGRAPH TO DAM #2

ISTAG	ICOMP	IECON	ITAPE	JPLT	JPRT	INAME	ISTAGE	IAUTO
3	0	0	0	0	3	1	0	0

HYDROGRAPH DATA

INHYD	IUMG	TAREA	SNAP	TRSDA	TRSPC	RATIO	ISHOW	ISAME	LOCAL
1	2	.06	.00	.06	1.00	.000	0	0	0

PRECIP DATA

SPTF	PHS	R6	R12	R24	R48	R72	R96
.00	25.70	102.00	120.00	130.00	.00	.00	.00

LOSS DATA

LROPT	STRKR	DLTKR	RTIOL	ENAIN	STRKS	RTIOK	STRTL	CMSTL	ALSMX	RTIMP
0	.00	.00	1.00	.00	.00	1.00	-1.00	-78.00	.00	.12

CURVE NO = -78.00 WETNESS = -1.00 EFFECT CM = 78.00

UNIT HYDROGRAPH DATA

TC	LAG
.05	.03

RECESSION DATA

STRTO	ORCSN	RTIOR
.00	-.10	2.00

TIME INCREMENT TOO LARGE--(NH0 IS 6T LAG/2)

UNIT HYDROGRAPH 5 END OF PERIOD OPINATES, TC = .05 HOURS, LAG = .03 VOL = 1.00

316. 89. 17. 3. 0.

MO.DA	HR.MN	PERIOD	RAIN	EXCS	LOSS	COMP	Q	END-OF-PERIOD FLOW	MO.DA	HR.MN	PERIOD	RAIN	EXCS	LOSS	COMP	Q
1.01	.05	1	.61	.00	.01	1.	1.01	12.05	145	.22	.19	.02	.68.			

1.01	10	2	.01	.00	.01	1.	1.01	12.10	146	.22	.20	.02	P3.
1.01	15	3	.01	.00	.01	1.	1.01	12.15	147	.22	.20	.02	P3.
1.01	20	4	.01	.00	.01	1.	1.01	12.20	148	.22	.20	.02	P4.
1.01	25	5	.01	.00	.01	1.	1.01	12.25	149	.22	.20	.02	85.
1.01	30	6	.01	.00	.01	1.	1.01	12.30	150	.22	.20	.02	85.
1.01	35	7	.01	.00	.01	1.	1.01	12.35	151	.22	.20	.02	85.
1.01	40	8	.01	.00	.01	1.	1.01	12.40	152	.22	.20	.02	86.
1.01	45	9	.01	.00	.01	1.	1.01	12.45	153	.22	.20	.02	86.
1.01	50	10	.01	.00	.01	1.	1.01	12.50	154	.22	.20	.02	86.
1.01	55	11	.01	.00	.01	1.	1.01	12.55	155	.22	.20	.01	87.
1.01	1.00	12	.01	.00	.01	1.	1.01	13.00	156	.22	.20	.01	87.
1.01	1.05	13	.01	.00	.01	1.	1.01	13.05	157	.26	.25	.02	100.
1.01	1.10	14	.01	.00	.01	1.	1.01	13.10	158	.26	.25	.02	104.
1.01	1.15	15	.01	.00	.01	1.	1.01	13.15	159	.26	.25	.01	105.
1.01	1.20	16	.01	.00	.01	1.	1.01	13.20	160	.26	.25	.01	106.
1.01	1.25	17	.01	.00	.01	1.	1.01	13.25	161	.26	.25	.01	106.
1.01	1.30	18	.01	.00	.01	1.	1.01	13.30	162	.26	.25	.01	106.
1.01	1.35	19	.01	.00	.01	1.	1.01	13.35	163	.26	.25	.01	106.
1.01	1.40	20	.01	.00	.01	1.	1.01	13.40	164	.26	.25	.01	107.
1.01	1.45	21	.01	.00	.01	1.	1.01	13.45	165	.26	.25	.01	107.
1.01	1.50	22	.01	.00	.01	1.	1.01	13.50	166	.26	.25	.01	107.
1.01	1.55	23	.01	.00	.01	1.	1.01	13.55	167	.26	.25	.01	107.
1.01	2.00	24	.01	.00	.01	1.	1.01	14.00	168	.26	.25	.01	107.
1.01	2.05	25	.01	.00	.01	1.	1.01	14.05	169	.33	.32	.01	127.
1.01	2.10	26	.01	.00	.01	1.	1.01	14.10	170	.33	.32	.01	133.
1.01	2.15	27	.01	.00	.01	1.	1.01	14.15	171	.33	.32	.01	135.
1.01	2.20	28	.01	.00	.01	1.	1.01	14.20	172	.33	.32	.01	135.
1.01	2.25	29	.01	.00	.01	1.	1.01	14.25	173	.33	.32	.01	135.
1.01	2.30	30	.01	.00	.01	1.	1.01	14.30	174	.33	.32	.01	135.
1.01	2.35	31	.01	.00	.01	1.	1.01	14.35	175	.33	.32	.01	136.
1.01	2.40	32	.01	.00	.01	1.	1.01	14.40	176	.33	.32	.01	136.
1.01	2.45	33	.01	.00	.01	1.	1.01	14.45	177	.33	.32	.01	136.
1.01	2.50	34	.01	.00	.01	1.	1.01	14.50	178	.33	.32	.01	136.
1.01	2.55	35	.01	.00	.01	1.	1.01	14.55	179	.33	.32	.01	136.
1.01	3.00	36	.01	.00	.01	1.	1.01	15.00	180	.33	.32	.01	136.
1.01	3.05	37	.01	.00	.01	1.	1.01	15.05	181	.20	.19	.00	97.
1.01	3.10	38	.01	.00	.01	1.	1.01	15.10	182	.40	.39	.01	147.
1.01	3.15	39	.01	.00	.01	1.	1.01	15.15	183	.40	.39	.01	162.
1.01	3.20	40	.01	.00	.01	1.	1.01	15.20	184	.60	.59	.01	227.
1.01	3.25	41	.01	.00	.01	1.	1.01	15.25	185	.70	.68	.01	277.
1.01	3.30	42	.01	.00	.01	1.	1.01	15.30	186	1.09	1.07	.03	599.
1.01	3.35	43	.01	.00	.01	1.	1.01	15.35	187	2.79	2.75	.04	1032.
1.01	3.40	44	.01	.00	.01	1.	1.01	15.40	188	1.10	1.08	.01	618.
1.01	3.45	45	.01	.00	.01	1.	1.01	15.45	189	.70	.69	.01	368.
1.01	3.50	46	.01	.00	.01	1.	1.01	15.50	190	.60	.59	.01	277.
1.01	3.55	47	.01	.00	.01	1.	1.01	15.55	191	.40	.39	.00	193.
1.01	4.00	48	.01	.00	.01	1.	1.01	16.00	192	.40	.39	.00	172.
1.01	4.05	49	.01	.00	.01	1.	1.01	16.05	193	.31	.30	.00	140.
1.01	4.10	50	.01	.00	.01	1.	1.01	16.10	194	.31	.30	.00	131.
1.01	4.15	51	.01	.00	.01	1.	1.01	16.15	195	.31	.30	.00	129.
1.01	4.20	52	.01	.00	.01	1.	1.01	16.20	196	.31	.30	.00	129.
1.01	4.25	53	.01	.00	.01	1.	1.01	16.25	197	.31	.30	.00	129.
1.01	4.30	54	.01	.00	.01	1.	1.01	16.30	198	.31	.30	.00	129.
1.01	4.35	55	.01	.00	.01	1.	1.01	16.35	199	.31	.30	.00	129.
1.01	4.40	56	.01	.00	.01	1.	1.01	16.40	200	.31	.30	.00	129.
1.01	4.45	57	.01	.00	.01	2.	1.01	16.45	201	.31	.30	.00	129.

1.01	4.50	58	.01	.00	.01	2.	1.01	16.50	202	.31	.30	.00	129.
------	------	----	-----	-----	-----	----	------	-------	-----	-----	-----	-----	------

1.01	9.30	114	.06	.05	.01	22.	1.01	21.30	258	.02	.00	9.
1.01	9.35	115	.06	.05	.01	22.	1.01	21.35	259	.02	.00	9.
1.01	9.40	116	.06	.05	.01	22.	1.01	21.40	260	.02	.00	9.
1.01	9.45	117	.06	.05	.01	22.	1.01	21.45	261	.02	.00	9.
1.01	9.50	118	.06	.05	.01	22.	1.01	21.50	262	.02	.00	9.
1.01	9.55	119	.06	.05	.01	22.	1.01	21.55	263	.02	.00	9.
1.01	10.00	120	.06	.05	.01	23.	1.01	22.00	264	.02	.00	9.
1.01	10.05	121	.06	.05	.01	23.	1.01	22.05	265	.02	.00	9.
1.01	10.10	122	.06	.05	.01	23.	1.01	22.10	266	.02	.00	9.
1.01	10.15	123	.06	.05	.01	23.	1.01	22.15	267	.02	.00	9.
1.01	10.20	124	.06	.05	.01	23.	1.01	22.20	268	.02	.00	9.
1.01	10.25	125	.06	.05	.01	23.	1.01	22.25	269	.02	.00	9.
1.01	10.30	126	.06	.05	.01	23.	1.01	22.30	270	.02	.00	9.
1.01	10.35	127	.06	.05	.01	23.	1.01	22.35	271	.02	.00	9.
1.01	10.40	128	.06	.05	.01	23.	1.01	22.40	272	.02	.00	9.
1.01	10.45	129	.06	.05	.01	23.	1.01	22.45	273	.02	.00	9.
1.01	10.50	130	.06	.06	.01	23.	1.01	22.50	274	.02	.00	9.
1.01	10.55	131	.06	.06	.01	23.	1.01	22.55	275	.02	.00	9.
1.01	11.00	132	.06	.06	.01	24.	1.01	23.00	276	.02	.00	9.
1.01	11.05	133	.06	.06	.01	24.	1.01	23.05	277	.02	.00	9.
1.01	11.10	134	.06	.06	.01	24.	1.01	23.10	278	.02	.00	9.
1.01	11.15	135	.06	.06	.01	24.	1.01	23.15	279	.02	.00	9.
1.01	11.20	136	.06	.06	.01	24.	1.01	23.20	280	.02	.00	9.
1.01	11.25	137	.06	.06	.01	24.	1.01	23.25	281	.02	.00	9.
1.01	11.30	138	.06	.06	.01	24.	1.01	23.30	282	.02	.00	9.
1.01	11.35	139	.06	.06	.01	24.	1.01	23.35	283	.02	.00	9.
1.01	11.40	140	.06	.06	.01	24.	1.01	23.40	284	.02	.00	9.
1.01	11.45	141	.06	.06	.01	24.	1.01	23.45	285	.02	.00	9.
1.01	11.50	142	.06	.06	.01	24.	1.01	23.50	286	.02	.00	9.
1.01	11.55	143	.06	.06	.01	24.	1.01	23.55	287	.02	.00	9.
1.01	12.00	144	.06	.06	.01	24.	1.02	.00	288	.02	.00	9.

SUM 33.41 30.03 2.78 13994.
 (849.)(778.)(71.)(396.27)

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
10.26	151.	49.	49.	13976.
29.	4.	1.	1.	396.
	25.56	32.83	32.83	32.83
	649.19	833.88	833.88	833.88
	75.	96.	96.	96.
	92.	119.	119.	119.

HYDROGRAPH AT STA 3 FOR PLAN 1, RTIO 1

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
103.	15.	5.	5.	1398.
3.	0.	0.	0.	40.
	2.56	3.28	3.28	3.28
	64.92	83.39	83.39	83.39
	7.	10.	10.	10.
	9.	12.	12.	12.

HYDROGRAPH AT STA 3 FOR PLAN 1, RTIO 2

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	206.	30.	10.	10.	2795.
CMS	6.	1.	0.	0.	79.
INCHES	5.11	6.57	6.57	6.57	6.57
MM	129.84	166.78	166.78	166.78	166.78
AC-FT	15.	19.	19.	19.	19.
THOUS CU M	18.	24.	24.	24.	24.

HYDROGRAPH AT STA 3 FOR PLAN 1, RTIO 3

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	258.	38.	12.	12.	3494.
CMS	7.	1.	0.	0.	99.
INCHES	6.39	8.21	8.21	8.21	8.21
MM	162.50	208.47	208.47	208.47	208.47
AC-FT	19.	24.	24.	24.	24.
THOUS CU M	23.	30.	30.	30.	30.

HYDROGRAPH AT STA 3 FOR PLAN 1, RTIO 4

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	310.	45.	15.	15.	4193.
CMS	9.	1.	0.	0.	119.
INCHES	7.67	9.85	9.85	9.85	9.85
MM	194.76	250.16	250.16	250.16	250.16
AC-FT	22.	29.	29.	29.	29.
THOUS CU M	28.	36.	36.	36.	36.

HYDROGRAPH AT STA 3 FOR PLAN 1, RTIO 5

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	361.	53.	17.	17.	4891.
CMS	10.	1.	0.	0.	139.
INCHES	8.95	11.49	11.49	11.49	11.49
MM	227.22	291.86	291.86	291.86	291.86
AC-FT	26.	34.	34.	34.	34.
THOUS CU M	32.	42.	42.	42.	42.

HYDROGRAPH AT STA 3 FOR PLAN 1, RTIO 6

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
CFS	413.	60.	19.	19.	5590.
CMS	12.	2.	1.	1.	158.
INCHES	10.22	13.13	13.13	13.13	13.13
MM	259.68	333.55	333.55	333.55	333.55

PEAK OUTFLOW IS 1796. AT TIME 14.60 HOURS

BEGIN DAM FAILURE AT 13.33 HOURS

PEAK OUTFLOW IS 1805. AT TIME 13.85 HOURS

***** SUB-AREA RUNOFF COMPUTATION *****

CALCULATF INFLOW HYDROGRAPH TO DAM #4

ISTAR ICOMP IECON ITAPE JPLT JPRT INAME ISTAGE IAUTO
 5 0 0 0 0 0 1 0 0

HYDROGRAPH DATA
 INYDG IUNG TAREA SNAP TRSDA TRSPC RATIO ISHOW ISAME LOCAL
 1 2 .09 .00 .09 1.00 .000 0 0 0 0

PRECIP DATA
 SPFE PMS R6 R12 R24 R48 R72 R96
 .00 25.70 102.00 120.00 130.00 .00 .00 .00

LOSS DATA
 LPROPT STRKR DLTKR RTIOL ERAIN STRKS RTIOK STRTL CMSTL ALSMX RTIMP
 0 .00 .00 1.00 .00 .00 1.00 -1.00 -81.00 .00 .00

CURVE NO = -81.00 WETNESS = -1.00 EFFECT CM = 81.00

UNIT HYDROGRAPH DATA
 TC= .00 LAG= .05

RECESSION DATA
 STRTQ= .00 QRESN= .00 RTIOR= 1.00

TIME INCREMENT TOO LARGE--(CMH) IS (T LAG/2)

UNIT HYDROGRAPH 5 END OF PERIOD ORDINATES, TC= .00 HOURS, LAG= .05 VOL= 1.00
 476. 182. 42. 10.

MO-DA	HR-MN	PERIOD	RAIN	EXCS	LOSS	END-OF-PERIOD FLOW	NO-DA	HR-MN	PERIOD	RAIN	EXCS	LOSS	COMP Q
1-01	.05	1	.01	.00	.01	0.	1-01	12-05	145	.22	.20	.02	108.
1-01	.10	2	.01	.00	.01	0.	1-01	12-10	146	.22	.20	.02	136.
1-01	.15	3	.01	.00	.01	0.	1-01	12-15	147	.22	.20	.02	140.
1-01	.20	4	.01	.00	.01	0.	1-01	12-20	148	.22	.20	.02	143.
1-01	.25	5	.01	.00	.01	0.	1-01	12-25	149	.22	.20	.02	143.
1-01	.30	6	.01	.00	.01	0.	1-01	12-30	150	.22	.20	.02	144.
1-01	.35	7	.01	.00	.01	0.	1-01	12-35	151	.22	.20	.02	145.
1-01	.40	8	.01	.00	.01	0.	1-01	12-40	152	.22	.20	.01	145.
1-01	.45	9	.01	.00	.01	0.	1-01	12-45	153	.22	.20	.01	146.

1.01	5.30	66	.01	.00	.01	3.	1.01	17.30	210	.24	.24	.00	170.
1.01	5.35	67	.01	.00	.01	3.	1.01	17.35	211	.24	.24	.00	170.
1.01	5.40	68	.01	.00	.01	3.	1.01	17.40	212	.24	.24	.00	170.
1.01	5.45	69	.01	.00	.01	3.	1.01	17.45	213	.24	.24	.00	170.
1.01	5.50	70	.01	.00	.01	3.	1.01	17.50	214	.24	.24	.00	170.
1.01	5.55	71	.01	.00	.01	3.	1.01	17.55	215	.24	.24	.00	170.
1.01	6.00	72	.01	.00	.01	3.	1.01	18.00	216	.24	.24	.00	170.
1.01	6.05	73	.06	.02	.04	12.	1.01	18.05	217	.02	.02	.00	27.
1.01	6.10	74	.06	.02	.04	16.	1.01	18.10	218	.02	.02	.00	27.
1.01	6.15	75	.06	.03	.04	18.	1.01	18.15	219	.02	.02	.00	18.
1.01	6.20	76	.06	.03	.04	20.	1.01	18.20	220	.02	.02	.00	16.
1.01	6.25	77	.06	.03	.04	21.	1.01	18.25	221	.02	.02	.00	15.
1.01	6.30	78	.06	.03	.03	22.	1.01	18.30	222	.02	.02	.00	15.
1.01	6.35	79	.06	.03	.03	23.	1.01	18.35	223	.02	.02	.00	15.
1.01	6.40	80	.06	.03	.03	23.	1.01	18.40	224	.02	.02	.00	15.
1.01	6.45	81	.06	.03	.03	23.	1.01	18.45	225	.02	.02	.00	15.
1.01	6.50	82	.06	.04	.03	23.	1.01	18.50	226	.02	.02	.00	15.
1.01	6.55	83	.06	.04	.03	26.	1.01	18.55	227	.02	.02	.00	15.
1.01	7.00	84	.06	.04	.03	26.	1.01	19.00	228	.02	.02	.00	15.
1.01	7.05	85	.06	.04	.03	27.	1.01	19.05	229	.02	.02	.00	15.
1.01	7.10	86	.06	.04	.02	28.	1.01	19.10	230	.02	.02	.00	15.
1.01	7.15	87	.06	.04	.02	28.	1.01	19.15	231	.02	.02	.00	15.
1.01	7.20	88	.06	.04	.02	28.	1.01	19.20	232	.02	.02	.00	15.
1.01	7.25	89	.06	.04	.02	30.	1.01	19.25	233	.02	.02	.00	15.
1.01	7.30	90	.06	.04	.02	30.	1.01	19.30	234	.02	.02	.00	15.
1.01	7.35	91	.06	.04	.02	31.	1.01	19.35	235	.02	.02	.00	15.
1.01	7.40	92	.06	.04	.02	31.	1.01	19.40	236	.02	.02	.00	15.
1.01	7.45	93	.06	.04	.02	31.	1.01	19.45	237	.02	.02	.00	15.
1.01	7.50	94	.06	.05	.02	31.	1.01	19.50	238	.02	.02	.00	15.
1.01	7.55	95	.06	.05	.02	32.	1.01	19.55	239	.02	.02	.00	15.
1.01	8.00	96	.06	.05	.02	33.	1.01	20.00	240	.02	.02	.00	15.
1.01	8.05	97	.06	.05	.02	33.	1.01	20.05	241	.02	.02	.00	15.
1.01	8.10	98	.06	.05	.02	33.	1.01	20.10	242	.02	.02	.00	15.
1.01	8.15	99	.06	.05	.02	33.	1.01	20.15	243	.02	.02	.00	15.
1.01	8.20	100	.06	.05	.02	34.	1.01	20.20	244	.02	.02	.00	15.
1.01	8.25	101	.06	.05	.02	34.	1.01	20.25	245	.02	.02	.00	15.
1.01	8.30	102	.06	.05	.02	35.	1.01	20.30	246	.02	.02	.00	15.
1.01	8.35	103	.06	.05	.01	35.	1.01	20.35	247	.02	.02	.00	15.
1.01	8.40	104	.06	.05	.01	35.	1.01	20.40	248	.02	.02	.00	15.
1.01	8.45	105	.06	.05	.01	36.	1.01	20.45	249	.02	.02	.00	15.
1.01	8.50	106	.06	.05	.01	36.	1.01	20.50	250	.02	.02	.00	15.
1.01	8.55	107	.06	.05	.01	36.	1.01	20.55	251	.02	.02	.00	15.
1.01	9.00	108	.06	.05	.01	36.	1.01	21.00	252	.02	.02	.00	15.
1.01	9.05	109	.06	.05	.01	36.	1.01	21.05	253	.02	.02	.00	15.
1.01	9.10	110	.06	.05	.01	37.	1.01	21.10	254	.02	.02	.00	15.
1.01	9.15	111	.06	.05	.01	37.	1.01	21.15	255	.02	.02	.00	15.
1.01	9.20	112	.06	.05	.01	37.	1.01	21.20	256	.02	.02	.00	15.
1.01	9.25	113	.06	.05	.01	37.	1.01	21.25	257	.02	.02	.00	15.
1.01	9.30	114	.06	.05	.01	36.	1.01	21.30	258	.02	.02	.00	15.
1.01	9.35	115	.06	.05	.01	36.	1.01	21.35	259	.02	.02	.00	15.
1.01	9.40	116	.06	.05	.01	36.	1.01	21.40	260	.02	.02	.00	15.
1.01	9.45	117	.06	.05	.01	38.	1.01	21.45	261	.02	.02	.00	15.
1.01	9.50	118	.06	.05	.01	38.	1.01	21.50	262	.02	.02	.00	15.
1.01	9.55	119	.06	.05	.01	38.	1.01	21.55	263	.02	.02	.00	15.
1.01	10.00	120	.06	.05	.01	39.	1.01	22.00	264	.02	.02	.00	15.
1.01	10.05	121	.06	.05	.01	39.	1.01	22.05	265	.02	.02	.00	15.

1.01	10.10	122	.06	.05	.01	39.	1.01	22.10	266	.02	.02	.00	15.
------	-------	-----	-----	-----	-----	-----	------	-------	-----	-----	-----	-----	-----

STATION	707.7	707.7	707.7	707.7	707.8	707.9	707.9	707.9	707.9
707.9	707.9	707.7	707.7	707.8	707.8	707.8	707.9	707.9	707.9
708.2	708.0	708.0	708.1	708.1	708.1	708.2	708.2	708.2	708.2
708.6	708.3	708.3	708.4	708.4	708.4	708.5	708.5	708.5	708.5
708.9	708.6	708.7	708.7	708.7	708.8	708.8	708.8	708.8	708.9
709.6	709.0	709.0	709.0	709.1	709.2	709.3	709.4	709.5	709.5
714.3	709.7	709.9	710.1	710.4	711.5	712.2	712.9	713.6	713.6
716.7	714.9	715.6	716.1	716.5	717.0	717.0	717.0	716.8	716.8
715.9	716.5	716.3	716.2	716.1	716.0	716.0	716.0	716.0	716.0
715.9	715.9	716.0	716.0	716.0	716.0	716.0	716.0	716.0	716.0
715.8	715.9	715.9	715.9	715.9	715.8	715.8	715.8	715.8	715.9
715.5	715.8	715.8	715.8	715.7	715.7	715.7	715.6	715.5	715.5
715.2	715.6	715.6	715.6	715.5	715.5	715.5	715.5	715.5	715.5
715.2	715.1	715.1	715.1	715.0	715.0	715.0	715.0	715.0	715.0
714.9	714.9	714.9	714.8	714.8	714.8	714.7	714.7	714.7	714.9
714.7	714.7	714.7	714.6	714.6	714.6	714.6	714.6	714.5	714.5
714.5	714.5	714.5	714.4	714.4	714.4	714.4	714.4	714.4	714.4
714.3	714.3	714.3	714.3	714.3	714.2	714.2	714.2	714.2	714.4
714.2	714.2	714.1	714.1	714.1	714.1	714.1	714.1	714.2	714.2

PEAK OUTFLOW IS 2617. AT TIME 15-07 HOURS

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
2617.	662.	189.	189.	54337.
74.	19.	5.	5.	1539.
	30.20	33.43	33.43	33.43
	767.07	849.12	849.12	849.12
	338.	374.	374.	374.
	477.	462.	462.	462.

PEAK FLOW AND STORAGE (END OF PERIOD) SUPPLY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)
 AREA IN SQUARE MILES (SQUARE KILOMETERS)

OPERATION	STATION	AREA	PLAN RATIO 1	RATIO 2	RATIO 3	RATIO 4	RATIO 5	RATIO 6	RATIO 7	RATIO 8	RATIO 9
			.10	.20	.25	.30	.35	.40	.50	.75	1.00
HYDROGRAPH AT	1	.06	1	117.	226.	339.	395.	451.	564.	646.	1129.
	(-.16)	(3.20)	6.30)	7.99)	11.19)	12.78)	15.98)	23.97)	31.96)
ROUTED TO	2	.06	1	1056.	1078.	1105.	1156.	1292.	1112.	1128.	1154.
	(-.16)	(.30)	29.90)	30.55)	32.73)	36.59)	31.49)	31.95)	32.67)

HYDROGRAPH AT	3	(.06	103.	266.	258.	310.	361.	413.	516.	774.	1032.
			.14)	2.92)	5.84)	7.30)	8.77)	10.23)	11.69)	14.61)	21.91)	29.22)
ROUTED TO	4	(.06	7.	17.	20.	1704.	1748.	1787.	1960.	1777.	1783.
			.14)	.21)	.47)	.56)	48.23)	49.50)	50.59)	55.50)	50.31)	50.49)
HYDROGRAPH AT	5	(.09	165.	331.	413.	496.	579.	661.	827.	1240.	1653.
			.24)	6.08)	9.36)	11.70)	14.04)	16.38)	18.72)	23.40)	35.11)	46.81)
3 COMBINED	6	(.21	177.	1115.	1151.	2670.	2783.	1973.	2413.	2421.	3197.
			.54)	5.02)	31.57)	32.58)	75.62)	78.80)	55.86)	68.33)	68.56)	96.52)
ROUTED TO	7	(.21	14.	65.	96.	2075.	2266.	1816.	2259.	1883.	2617.
			.54)	.40)	1.86)	2.71)	58.73)	66.17)	51.42)	63.96)	53.33)	74.10)

***** NO NEW SAFETY ANALYSIS *****

SUMMARY OF DAM SAFETY ANALYSIS

DAM # 1

PLAN 1

INITIAL VALUE 748.40 SPILLWAY CREST 748.40 TOP OF DAM 751.30
 ELEVATION STORAGE 42.0
 OUTFLOW 0.0

RATIO OF PMF	MAXIMUM RESERVOIR U.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
.10	750.66	.00	48.	11.	.00	18.68	.00
.20	751.34	.04	53.	1062.	.23	16.69	15.92
.25	751.63	.33	55.	1083.	.33	16.44	15.67
.30	751.75	.45	55.	1109.	.37	16.29	15.50
.35	751.62	.32	55.	1157.	.42	16.04	15.33
.40	751.39	.09	53.	1332.	.27	15.65	14.83
.50	751.40	.10	54.	1122.	.29	14.96	14.17
.75	751.44	.14	54.	1138.	.31	13.96	13.17
1.00	751.45	.15	54.	1165.	.31	13.37	12.58

SUPMARY OF DAM SAFETY ANALYSIS
 DAM # 2

B L A C K & V E A T C H
 FLOOD HYDROGRAPH PACKAGE - MEC-1

PLAN 1	ELEVATION STORAGE OUTFLOW	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
	775.60	773.60	777.10	777.10						
	55.	55.	70.	70.						
	0.	0.	20.	20.						
RATIO OF PMF	MAXIMUM RESERVOIR U.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS			
.10	775.04	.60	61.	7.	.20	16.42	.00			
.20	776.34	.00	67.	17.	.00	16.25	.00			
.25	777.05	.00	70.	20.	.20	16.33	.00			
.30	777.18	.68	71.	1706.	.21	16.33	15.83			
.35	777.47	.37	72.	1769.	.29	16.19	15.67			
.40	777.68	.58	73.	1801.	.31	16.10	15.58			
.50	777.59	.29	72.	1995.	.35	15.85	15.33			
.75	777.27	.17	71.	1796.	.25	14.60	14.08			
1.00	777.28	.18	71.	1805.	.27	13.85	13.33			

B L A C K & V E A T C H
 FLOOD HYDROGRAPH PACKAGE - MEC-1

SUPMARY OF DAM SAFETY ANALYSIS

SUPMARY OF DAM SAFETY ANALYSIS

DAM # 4

PLAN 1	ELEVATION STORAGE OUTFLOW	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM	DURATION OVER TOP MAJ OUTFLOW HOURS	TIME OF FAILURE HOURS
		707.00	707.00	716.40		
		118.	118.	200.		
		0.	0.	66.		
RATIO OF PHF	MAXIMUM RESERVOIR M.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP MAJ OUTFLOW HOURS	TIME OF FAILURE HOURS
.10	706.79	.00	136.	14.	.00	.00
.20	714.28	.00	198.	65.	.00	.00
.25	716.91	.51	206.	96.	3.50	17.25
.30	717.33	2.73	236.	2075.	4.08	16.50
.35	717.82	2.82	237.	2266.	4.50	16.33
.40	717.00	2.60	234.	1816.	5.00	16.17
.50	717.22	2.82	237.	2259.	5.83	15.92
.75	717.03	2.63	234.	1883.	7.58	15.67
1.00	717.39	2.99	239.	2817.	8.83	15.67

SEQ # CTS # 1 2 3 4 5 6 7 8

51 88748.4
 52 30751.3
 53 85.
 54 30751.3 751.4 180. 305. 330. 370. 415. 470.
 55 3810. 0.5 724.7 1. 748.4 751.3 754. 755.
 56 K

57 K1 CALCULATE INFLOW HYDROGRAPH TO DAM #2

CT	1	2	3	4	5	6	7	8
58	0	288	7.464	20.055	1.60	.50		
59	01	.007	.007	.007	.007	.007	.007	.007
60	01	.007	.007	.007	.007	.007	.007	.007
61	01	.007	.007	.007	.007	.007	.007	.007
62	01	.007	.007	.007	.007	.007	.007	.007
63	01	.007	.007	.007	.007	.007	.007	.007
64	01	.007	.007	.007	.007	.007	.007	.007
65	01	.007	.007	.007	.007	.007	.007	.007
66	01	.007	.007	.007	.007	.007	.007	.007
67	01	.007	.007	.007	.007	.007	.007	.007
68	01	.013	.013	.013	.013	.013	.013	.013
69	01	.013	.013	.013	.013	.013	.013	.013
70	01	.013	.013	.013	.013	.013	.013	.013
71	01	.026	.026	.026	.026	.026	.026	.026
72	01	.026	.026	.026	.026	.026	.026	.026
73	01	.029	.029	.029	.029	.029	.029	.029
74	01	.124	.124	.124	.124	.124	.124	.124
75	01	.081	.081	.081	.081	.081	.081	.081
76	01	.029	.029	.026	.026	.026	.026	.026
77	01	.026	.026	.026	.026	.026	.026	.026
78	01	.013	.013	.013	.013	.013	.013	.013
79	01	.013	.013	.013	.013	.013	.013	.013
80	01	.013	.013	.013	.013	.013	.013	.013
81	01	.013	.013	.013	.013	.013	.013	.013
82	01	.007	.007	.007	.007	.007	.007	.007
83	01	.007	.007	.007	.007	.007	.007	.007
84	01	.007	.007	.007	.007	.007	.007	.007
85	01	.007	.007	.007	.007	.007	.007	.007
86	01	.007	.007	.007	.007	.007	.007	.007
87	01	.007	.007	.007	.007	.007	.007	.007
88	01	.007	.007	.007	.007	.007	.007	.007
89	01	.007	.007	.007	.007	.007	.007	.007
90	420.05	0.03						0.12
91	A	-0.1						
92	K	1	4	1				
93	K1	ROUTE	HYDROGRAPH	THROUGH	DAM	#2		
94	V	1						
95	V1	1			55.			
96	Y	74773.6	774.0	775.0	776.0	777.1	779.0	
97	Y1	2.0	7.0	15.0	20.0	21.0		
98	Y1	6.	18.	42.	55.	63.		
99	Y1	750.	750.	760.	770.	773.6	780.	
100	Y1							

90 420.05 0.03
 91 A -0.1
 92 K 1 4 1
 93 K1 ROUTE HYDROGRAPH THROUGH DAM #2
 94 V 1
 95 V1 1
 96 Y 74773.6 774.0 775.0 776.0 777.1 779.0
 97 Y1 2.0 7.0 15.0 20.0 21.0
 98 Y1 6. 18. 42. 55. 63.
 99 Y1 750. 750. 760. 770. 773.6 780.
 100

SEQ # CTS # 1 2 3 4 5 6 7 8

-----CARD COLUMNS-----
 1 2 3 4 5 6 7 8
 SEQ # CTS # 123456789012345678901234567890123456789012345678901234567890
 151 SAC-0 2.7 8.8 15.2 18.1
 152 S676.5 690. 705. 720. 730.
 153 S8707.
 154 S0774.4
 155 SAC-0 53. 89. 161. 215. 276. 386. 430. 441. 465.
 156 S8714.4 714.8 715.1 715.8 716. 716.5 716.8 717.6 719. 720.9
 157 R 99

0 1 2 3 4 5 6 7 8
 SEQ # CTS # 1234567890123456789012345678901234567890123456789012345678901234567890
 BASG,A SYSLIB=ADD. 040200104000
 FAC WARNING

BASG,T 2.
 BASG,T 3.
 BASG,T 4.
 BASG,T 7.
 BASG,T 10.
 BASG,T 11.
 BASG,T 13.
 BASG,T 14.
 BASG,T 15.

BASG,A SYSLIB=M21. C40200004000
 FAC WARNING
 BASG,A SYSLIB=UC7. 040200104000
 FAC WARNING

BXQT SYSLIB=UC7.U07
 BXDT,V SYSLIB=M21.M21/02-2

THOUS CU M 9. 16. 16. 16.

RUNOFF SUMMARY, AVERAGE FLOW IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)
 AREA IN SQUARE MILES (SQUARE KILOMETERS)

	PEAK	6-HOUR	24-HOUR	72-HOUR	AREA
HYDROGRAPH AT 1	180.	19.	6.	6.	.06
	(5.11)(.54)(.17)(.17)(.16)
ROUTED TO 2	11.	16.	4.	4.	.06
	(.32)(.26)(.12)(.12)(.16)
HYDROGRAPH AT 3	169.	17.	5.	5.	.06
	(4.79)(.48)(.15)(.15)(.14)
ROUTED TO 4	7.	7.	3.	3.	.66
	(.21)(.20)(.09)(.09)(.14)
HYDROGRAPH AT 5	274.	28.	8.	8.	.09
	(7.75)(.78)(.23)(.23)(.24)
3-COMBINED 6	282.	43.	16.	16.	.21
	(7.97)(1.22)(.45)(.45)(.54)
ROUTED TO 7	15.	15.	6.	6.	.21
	(.43)(.42)(.18)(.18)(.54)

AD-A107 689

BLACK AND VEATCH KANSAS CITY MO
NATIONAL DAM SAFETY PROGRAM. BURTON-DUENKE DAM NUMBER 4 (MO 317--ETC(U)
APR 81 E R BURTON, H L CALLAHAN
DACW43-81-C-0037
NL

F/6 13/13

UNCLASSIFIED

2 of 2

ADA
107689



			END
			DATE
			FILED
			1 1982
			DTIC

SUMMARY OF DAM SAFETY ANALYSIS

DAM # 1

100-yr

PLAN 1

INITIAL VALUE SPILLWAY CREST TOP OF DAM
 748.40 751.30
 ELEVATION STORAGE 42. 53.
 OUTFLOW 0. 24.

RATIO OF PHF	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	0.00	48. 11.	0.00	13.67	0.00	0.00

SUMMARY OF DAM SAFETY ANALYSIS

B L A C K V E A T C H
 FLOOD HYDROGRAPH PACKAGE - HEC-1

PROJECT 9457: DATE 14 MAY 81
 PROGRAM H21/02-2V TIME 10:01:47 CASE #4

SUPMARY OF DAM SAFETY ANALYSIS

DAM # 4

100-yr

PLAN 1

INITIAL VALUE SPILLWAY CREST TOP OF DAM
 707.00 714.40
 ELEVATION STORAGE 118. 200.
 OUTFLOW 0. 66.

RATIO OF PHF	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	.00	136.	15.	.00	22.25	.00

DATE
ILME