VALIDATION OF FINITE SEGMENT CABLE MODELS. (U)

R L HUSTON, J W KAMMAN

APR 81

N00014-76-C-013
Analytical and experimental data is presented validating a finite segment cable model. The model consists of a series of pin-connected rigid rods which may have different lengths, diameters, and masses. The model is capable of simulating large, three-dimensional motion of flexible cables. Its principal areas of application are expected to be with the simulation of long, heavy, towing and hoisting cables.
VALIDATION OF FINITE SEGMENT CABLE MODELS

Ronald L. Huston
and
James W. Kamman

Department of Mechanical and Industrial Engineering
Mail Location No. 72
University of Cincinnati
Cincinnati, Ohio 45221

This Technical Report was prepared with support of
Office of Naval Research
Contract N00014-76C-0139
ABSTRACT

Analytical and experimental data is presented validating a finite segment cable model. The model consists of a series of pin-connected rigid rods which may have different lengths, diameters, and masses. The model is capable of simulating large, three-dimensional motion of flexible cables. Its principal areas of application are expected to be with the simulation of long, heavy, towing and hoisting cables.
Approximately five years ago we introduced a three-dimensional dynamic finite segment model for cables and chains [1]. The model consists of a series of pin-connected rods or links. The model is developed so that arbitrary external forces may be exerted on the links. The dynamics of the system is then determined through numerical integration of the governing equations of motion. In this report we present data which validates analytical predictions of the model.

Figure 1 shows a schematic representation of the model. The number of links N is arbitrary. The length, diameter, mass, and inertias of the individual links may also be arbitrarily chosen. This model is expected to be effective in studying the nonlinear, three-dimensional, dynamic behavior of long, heavy cables [1, 2]. Particular application with submerged towing cables has also been suggested [3].

The attractive features of the model are: 1) the arbitrary dimensions and physical parameters of the links of the model; 2) the arbitrary specification of externally applied forces; 3) the use of relative orientation angles between the links to define the system configuration; and 4) the use of Lagrange's form of d'Alembert's principle to develop the governing equations of motion. The use of relative orientation angles is a convenience in the specification of the system's configuration and in the introduction of

* Numbers in brackets refer to References at the end of the report.
flexible and/or torsional springs and dampers between the links.

Lagrange's form of d'Alembert's principle as exposited by Kane and others [4-8], has been shown, for large systems, to possess the advantages of both Lagrange's equations and Newton's laws, but without the corresponding disadvantages. That is, the principle provides for the automatic elimination of the "non-working" internal constraint forces without introducing tedious differentiation or other similar calculations.

The objective of this report is to present several sets of data validating this cable model. This data consists of: 1) a comparison of results obtained from this model with analogous results obtained from a two-dimensional multi-link pendulum model with governing equations developed by Lagrange's equations; 2) a comparison of data from the above models with the displacement and natural frequencies of a hanging cable with data obtained analytically from a linear partial differential equation model; and 3) a comparison of model data for a submerged pendulum with experimental data recorded at the Civil Engineering Laboratory at Port Hueneme, California.

The balance of this report is divided into four parts with the following part summarizing the basic equations of the kinematics and dynamics of the cable model. This is followed by the development of the two-dimensional Lagrange multi-link pendulum model. The comparisons of the models with each other and with analytical and experimental data are presented in the next part. The final part contains some concluding remarks on the significance of the validation and on the application of the model.
THE FINITE SEGMENT MODEL

Configuration

Consider again the representation of the model as shown in Figure 1. To establish a geometrical accounting for this system, select one link (say the first link) as the reference link. Next, label or number the other links in ascending progression away from this link as shown in Figure 1. The configuration and kinematics of each body of this system may then be developed relative to the reference link which, in turn, has its configuration and kinematics defined relative to an inertial reference frame R.

Consider a typical pair of adjoining links such as $L_j$ and $L_k$ as shown in Figure 2. Let $j<k$, that is, let $j=k-1$. Then the general orientation of $L_k$ relative to $L_j$, may be defined in terms of the relative inclination of the dextral orthogonal unit vector sets, $\mathbf{n}_{ji}$ and $\mathbf{n}_{ki}$ (i=1,2,3) fixed in $L_j$ and $L_k$, as shown in Figure 2. Specifically, let $L_j$ and $L_k$ be oriented so that $\mathbf{n}_{ji}$ and $\mathbf{n}_{ki}$ are respectively parallel. Then $L_k$ may be brought into any given orientation relative to $L_j$ by three successive dextral rotations about axes parallel to $\mathbf{n}_{kl}$, $\mathbf{n}_{kl'}$, and $\mathbf{n}_{kl''}$ through the angles $\alpha$, $\beta$, and $\gamma$. $\mathbf{n}_{ji}$ and $\mathbf{n}_{ki}$ are then related to each other as:

$$\mathbf{n}_{ji} = \mathbf{SK}_{jm}\mathbf{n}_{km}$$  (1)
Figure 1. The Finite Segment Cable Model
where \( SK \) is a 3x3 orthogonal transformation matrix ("shifter") defined as [9]:

\[
SK_{im} = \delta_{ij} \cdot \delta_{km}
\]  

(Regarding notation, repeated subscripts, such as \( m \) in the right side of Equation (1) represent a sum over the range \( (1,2,3) \) of that index.)

\( SK \) may be written as the product of three orthogonal matrices as:

\[
SK = AK \cdot BK \cdot GK 
\]  

where

\[
AK = \begin{bmatrix}
1 & 0 & 0 \\
0 & C_{\alpha_k} & -S_{\alpha_k} \\
0 & S_{\alpha_k} & C_{\alpha_k}
\end{bmatrix}
\]  

(4)

\[
BK = \begin{bmatrix}
C_{\beta_k} & 0 & S_{\beta_k} \\
0 & 1 & 0 \\
-S_{\beta_k} & 0 & C_{\beta_k}
\end{bmatrix}
\]  

(5)

\[
GK = \begin{bmatrix}
C_{\gamma_k} & -S_{\gamma_k} & 0 \\
S_{\gamma_k} & C_{\gamma_k} & 0 \\
0 & 0 & 1
\end{bmatrix}
\]  

(6)

where \( S \) and \( C \) represent the sine and cosine.
These expressions allow for the transformation of components of vectors referred to one link of the system into components referred to any other link of the system, and, in particular, to the inertial reference frame R.

Since these transformation matrices play a central role throughout the analysis, it is helpful to also have an algorithm for this derivative, especially the derivative of $S_{OK}$, the transformation matrix between $n_{ki}$ and $n_{oi}$ where the $n_{oi}$ are fixed in R. Specifically, $S_{OK}$ is

$$S_{OK_{ij}} = n_{oi} \cdot n_{kj}$$  \hspace{1cm} (7)

Hence, since the $n_{oi}$ are fixed and therefore constant in R, the following is obtained:

$$\frac{d}{dt}(S_{OK_{ij}}) = \frac{d}{dt}(n_{oi} \cdot n_{kj})$$  \hspace{1cm} (8)

where the superscript R indicates that the derivative is computed in R. However, since $n_{kj}$ are fixed in $L_k$, their derivative may be written as $\omega_k \times n_{ki}$ where $\omega_k$ is the angular velocity of $L_k$ in R. Equation (8) may then be rewritten as:

$$\frac{d}{dt}(S_{OK_{ij}}) = -\epsilon_{imn} k_m o_{om} \cdot n_{kj}$$  \hspace{1cm} (9)

or as

$$\frac{d}{dt}(S_{OK}) = \omega K S_{OK}$$  \hspace{1cm} (10)
where \( WK \) is a matrix defined as

\[
WK_{im} = -e_{imn} \omega_{kn}
\]

(11)

where \( \omega_{kn} \) are the \( n \) components of \( \omega_k \) and \( e_{imn} \) is the standard permutation symbol [9,10]. \( WK \) is simply the matrix whose dual vector [10] is \( \omega_k \). Equation (10) thus shows that the derivative may be computed by a simple matrix multiplication.

Finally, Figure 2. contains symbols not yet defined in the sequel. \( 0_j \) \((j=1,\ldots,N)\) is the reference point of \( L_j \), and it is the common point of \( L_j \) and its adjacent lower-numbered link. \( G_j \) \((j=1,\ldots,N)\) represents the mass center of \( L_j \). \( \xi_j \) \((j=1,\ldots,N)\) is the position vector of \( G_j \) relative to \( 0_j \), and \( \xi_k \) is the position vector of \( 0_k \) relative to \( 0_j \). \( \xi_j \) and \( \xi_k \) are thus fixed in \( L_j \).

**Kinematics**

The system shown in Figure 1. will in general have \( 3N+3 \) degrees of freedom. These may be defined in terms of generalized coordinates \( X_j \) \((j=1,\ldots,3N+3)\). \( X_1, X_2, \) and \( X_3 \) represent the position coordinates of \( O_1 \) in \( R \). The succeeding triplets of coordinates represent dextral rotations of the links relative to the respective adjacent lower links.

If the axial rotation of the links is neglected, the number of degrees of freedom can be reduced to \( 2N+3 \). This also avoids singularities which are occasionally encountered with large rotations as discussed in References [11] and [12]. When the axial rotations are neglected,
the \( \dot{\alpha}_k \) are zero for each link and the BK become identity matrices. In the following kinematic analysis, the axial rotations of the links are neglected.

The angular velocity \( \omega_k \) of \( L_k \) in \( R \) is readily obtained from the addition formula [6]:

\[
\omega_k = \dot{\omega}_1 + \dot{\omega}_2 + \ldots + \dot{\omega}_k
\]  

where \( \dot{\omega}_k \) is the angular velocity of \( L_k \) relative to \( L_{k-1} \). By using the transformation properties of the shifters, \( \dot{\omega}_k \) may be written as:

\[
\dot{\omega}_k = \text{SOJ}_{im} (\dot{\gamma}_k \delta_{m\dot{i}} + \gamma_k \text{AK}_m3) n_i
\]  

where \( \delta_{mn} \) is Kronecker's delta symbol or the identity tensor [9,10]. Hence, by repeatedly substituting from Equation (13) into Equation (12), \( \omega_k \) takes the form:

\[
\omega_k = \omega_{kjm} \dot{X}_n\dot{m} n_m
\]  

where there is a sum from 1 to \( 2N+3 \) on \( j \) and from 1 to 3 on \( m \). From Equation (13), it is seen that the non-zero \( \omega_{kjm} \) take one of the two forms:

\[
\omega_{kjm} = \text{SOJ}_{ml} \text{AK}_m n_j
\]  

depending upon whether \( X_j \) is \( \alpha_k \) or \( \gamma_k \).
The angular acceleration $\ddot{x}_k$ of $L_k$ in $R$ may be obtained by differentiating Equation (14). Noting that the $n_{om}$ are constant, this becomes:

$$\ddot{x}_k = (\omega_{k^{\prime}l}^\prime \ddot{x}_l + \dot{\omega}_{k^{\prime}l}^\prime \dot{x}_l)n_{om}$$

(16)

where from Equation (15) the non-zero $\dot{\omega}_{k^{\prime}l}^\prime$ take one of the two forms:

$$\dot{\omega}_{k^{\prime}l}^\prime = \frac{S\omega_{j^{\prime}l}^{\prime} \omega_{m^{\prime}l}^{\prime}}{S\omega_{mn^{\prime}}^{\prime} \omega_{n^{\prime}l}^{\prime} + S\omega_{mn^{\prime}}^{\prime} \omega_{n^{\prime}l}^{\prime}}$$

(17)

where $S\omega$ is given by Equation (10) and where $\omega$ is obtained by differentiating Equation (4).

The velocity $V_j$ of $G_j$ in $R$ may be obtained by differentiating $P_j$, the position vector of $G_j$ relative to a fixed point $O$ in $R$. From Figure 1., $P_j$ may be expressed as:

$$P_j = (X_k + S\omega_{k^{\prime}l}^\prime P_j^{\prime} + \sum_{M=1}^{j-1} S\omega_{k^{\prime}l}^\prime P_j^{\prime})n_{om}$$

(18)

where, as before, there is a sum over $k$ and $l$ from 1 to 3. Hence, $V_j$ may be written in the form:

$$V_j = V_j^{\prime} \dot{x}_l^{\prime}n_{om}$$

(19)

where by Equations (13), (10), and (11), the non-zero $V_j^{\prime}$ are given by:

$$V_j^{\prime}k = \ddot{x}_l^{\prime}k \quad (j = 1, \ldots, N; \ \kappa = 1, 2, 3)$$

(20)
\[ V_{jik} = \sum_{j=1}^{M} W_{kpl}^j \tau_p + \sum_{M=1}^{W_{kpl}} \xi_{mp} \]  

where \( WJ_{kpl} \) is defined as:

\[ W_{kpl} = \left[ \sum_{j=1}^{2N+3} \frac{\partial \xi_{ij}}{\partial \tilde{X}_k} \right]_{SOJ} \]  

Using Equation (11), \( W_{kpl} \) may be written in the form:

\[ W_{kpl} = -e_{kqs} \omega_{jis} SOJ_{qp} \]  

The acceleration \( a_j \) of \( G_j \) in \( R \) may be obtained by differentiating Equation (19), leading to:

\[ a_j = (\ddot{V}_{jik} \dot{X}_k + \dot{V}_{jik} \ddot{X}_k)_{ok} \]  

where by Equations (20) and (21), the non-zero \( \dot{V}_{jik} \) are given by:

\[ \dot{V}_{jik} = \dot{W}_{kpl}^j \tau_p + \sum_{M=1}^{W_{kpl}} \check{\xi}_{mp} \]  

where by Equation (23) \( \dot{W}_{kpl}^j \) is:

\[ \dot{W}_{kpl}^j = -e_{kqs} (\omega_{jis} SOJ_{qp} + \omega_{jis} SOJ_{qp}) \]  

Therefore, the kinematical description of the system is defined by Equations (14), (16), (19), and (24), and specifically by the four block
matrices $\mathbf{J}_{jik}$, $\mathbf{V}_{jik}$, and $\mathbf{V}_{jik}$. From Equations (15), (17), (21), and (25), it is seen that each of these matrices may be computed by vector and matrix multiplications which are easily developed into computer algorithms. These matrices play a central role in the development of the equations of motion of the model.

Equations of Motion

Consider again the cable model of Figure 1. Let the externally applied force system on each link $L_k$ be replaced by an equivalent force system consisting of a single force $F_j$, passing through $G_j$ together with a couple with torque $M_j$. Then Lagrange's form of d'Alembert's principle states that the governing dynamical equations of motion for the chain system are:

$$ F_{2i} + F_{2*} = 0 \quad (i = 1, \ldots, 2N+3) \quad (27) $$

$F_{2i}$ are called "generalized active forces" and they are given by:

$$ F_{2i} = V_{jik} F_{jk} + \omega_{jik} M_{jk} \quad (28) $$

where there is a sum from 1 to N on j and from 1 to 3 on k, and where $F_{jk}$ and $M_{jk}$ are the $a_{0k}$ components of $F_j$ and $M_j$. $F_{2*}$ are called "generalized inertia forces" and they are given by:

$$ F_{2*} = V_{jik} F_{jk} + \omega_{jik} M_{jk} \quad (29) $$
where the indices follow the same rules as in Equation (28) and where $F^*_j$ and $M^*_j$ are the $n_{ok}$ components of the inertia forces $F^*_j$ and inertia torques $M^*_j$ given by the expressions [6]:

$$ F^*_j = -m_j \v a_j \quad \text{(no sum)} \quad (30) $$

and

$$ M^*_j = -I^j \cdot \v a_j - \omega_j \times (I^j \cdot \omega_j) \quad \text{(no sum)} \quad (31) $$

where $m_j$ is the mass of $L_j$ and $I^j$ is the inertia dyadic of $L_j$ relative to $G_j$ ($j=1,\ldots,N$).

By substituting Equations (16) and (24) into Equations (30) and (31) and ultimately into Equation (27), the equations of motion may be written in the form:

$$ a_{zp} = f_z \quad \left( \ell = 1,\ldots,2N+3 \right) \quad (32) $$

where there is a sum from 1 to $2N+3$ on $p$ and where $a_{zp}$ and $f_z$ are given by:

$$ a_{zp} = m_j v^j p^k v^k j l k + I^j k n^j p m^j n^j l k $$

and
where there is a sum from 1 to N on j, q, and s and a sum from 1 to 3 on the other repeated indices.

Equations (32) form a set of 2N+3 simultaneous ordinary, nonlinear differential equations determining the 2N+3 generalized coordinates $X_i$ of the cable system. Since the coefficients $a_{lp}$ and $f_i$, of these equations are algebraic functions of the physical parameters, and the four arrays $V_{jlk}$, $\dot{V}_{jlk}$, $\ddot{V}_{jlk}$ and $\omega_{jlk}$, the equations may be generated on a computer. Furthermore, once they are developed, they may also be solved numerically by a computer by using a standard numerical integration routine.

**Computer Code**

Numerical algorithms to evaluate the above parameters and expressions have been developed and compiled into a user-oriented computer code. As input, the code requires: the number of links; the masses; the centroidal principal inertia matrices; the mass center positions; the connection (or reference) point positions; the motion profile for those links with specified motion; the applied forces and moments; and the initial configuration.

The output of the code includes: the values of all variables and their first derivatives; the mass center positions, velocities, and accelerations; and the connection point positions, velocities, and accelerations.
LAGRANGE MULTI-LINK PENDULUM MODEL

To obtain an analytical verification of the governing equations (32), consider the two-dimensional oscillations of a multilink hanging pendulum with a concentrated end mass as shown in Figure 3. This system has $N$ degrees of freedom which may be described by the angles $\theta_1, \ldots, \theta_N$ as shown in Figure 3. The equations of motion of this relatively simple system can be obtained through Equations (32) or independently by using Lagrange's equations.

If each link has the same mass $m$ and length $l$ the kinetic energy $K$ of the system may be expressed as:

$$ K = \frac{1}{2} \left( \frac{m}{2} \left( \gamma_{G_1}^2 + \gamma_{G_2}^2 + \ldots + \gamma_{G_N}^2 + \left( l^2 / 12 \right) (\dot{\theta}_1^2 + \overline{\theta}_2^2 + \ldots + \overline{\theta}_N^2) \right) + \gamma_Q^2 \right) + \frac{M}{2} \overline{\gamma}_Q^2 \tag{35} $$

where the $G_i$ ($i = 1, \ldots, N$) are the mass centers of the links, $Q$ is the end point with mass $M$. The velocity of a typical mass center $G_i$ may be expressed as:

$$ \gamma_i = \dot{z}_i \hat{z}_1 + \dot{z}_2 \hat{z}_2 + \ldots + \dot{z}_N \hat{z}_N \tag{36} $$

where the $\hat{z}_i$ ($i = 1, \ldots, N$) are unit vectors normal to the links as shown in Figure 3. By substituting expressions of the form of Equation (36)
Figure 3. Two-Dimensional Multilink Pendulum Model
into Equation (35) and by carrying out the scalar products, the kinetic energy may be written in the form:

\[ K = \frac{1}{2} \sum_{i=1}^{N} \sum_{j=1}^{N} m_{ij} \dot{\theta}_i \dot{\theta}_j \]  

(37)

where the \( m_{ij} \) are given by the expressions:

\[ m_{ij} = \frac{1}{2} \left[ 1 + 2(N-k) + 2(M/m) \right] \cos(\dot{\theta}_j - \dot{\theta}_i) \]  

(38)

if \( i \neq j \) and \( k \) is the larger of \( i \) and \( j \)

and

\[ m_{ij} = [(N - i) + (1/3) + (M/m)] \]  

(39)

if \( i = j \)

Lagrange's equations then lead to governing equations of motion of the form [6]:

\[ d\left(\frac{\partial K}{\partial \dot{\theta}_i}\right)/dt - \frac{\partial K}{\partial \theta_i} = F_{\theta_i} \]  

(40)

where \( F_{\theta_i} \) is the generalized active force associated with \( \dot{\theta}_i \). The only forces making a non-zero contribution to \( F_{\theta_i} \) are the weight forces. Hence, \( F_{\theta_i} \) may be written as:

\[ F_{\theta_i} = \sum_{j=1}^{N} \left( \frac{\partial V}{\partial \dot{\theta}_j} \right) \cdot (-mgk) + \left( \frac{\partial V}{\partial \dot{\theta}_j} \right) \cdot (-Mg\kappa) \]  

(41)

where \( \kappa \) is the vertical unit vector as shown in Figure 3, and \( g \) is the gravitational constant.
By substituting Equation (37) and (41) into Equations (40), and by carrying out the indicated operations, the equations of motion take the form:

\[ \sum_{j=1}^{N} [m_{ij} \ddot{q}_j + p_{ij} \dot{q}_j^2 + \frac{g}{2} k_{ij}] = 0 \quad i = 1, \ldots, N \]  

(42)

where the \( m_{ij} \) are given in Equations (38) and (39) and where \( p_{ij} \) and \( k_{ij} \) are given by the expressions:

\[ p_{ij} = \frac{1}{2} \left[ 1 + 2(N-k) + 2(M/m) \right] \sin(q_i - q_j) \]  

(43)

if \( i \neq j \) and \( k \) is the larger of \( i \) and \( j \)

and

\[ p_{ij} = 0 \quad \text{if } i = j \]  

(44)

and

\[ k_{ij} = 0 \quad \text{if } i \neq j \]  

(45)

and, finally

\[ k_{ij} = \left[ N - i - \frac{L}{2} + (M/m) \right] \sin^2 \frac{q_i}{2} \quad \text{if } i = j \]  

(46)
MODEL VALIDATION AND COMPARISON

It is relatively easy to show - particularly if the number of links is small - that the equations of motion given by Equations (32) and (42) are the same. (This, of course, requires the conversion of the relative orientation angles used in Equations (32) to absolute angles as shown in Figure 3.) Hence, when Equations (32) and (42) were independently integrated for a number of cases as described below the results were identical.

The Hanging Chain

The small oscillations and natural frequencies of a hanging cable or chain have been examined and studied by several writers, including Salvadori and Schwartz [13] and Woodward [14]. These investigations involve the solution of the governing partial differential equation modelling the continuum of the hanging cable.

For an initially straight cable inclined at an angle \( \theta_0 \) with the vertical, and released from rest, the horizontal displacement \( y \) may be expressed approximately as [13]:

\[
y = y(x,t) = 88_0 L [0.139 J_0 (2.40 \sqrt{\frac{x}{L}}) \cos 1.20 \sqrt{\frac{g}{L}} t - 0.0175 J_0 (5.52 \sqrt{\frac{x}{L}}) \cos 2.76 \sqrt{\frac{g}{L}} t + 0.00568 J_0 (8.63 \sqrt{\frac{x}{L}}) \cos 4.33 \sqrt{\frac{g}{L}} t] \quad (47)
\]
where $x$ is the vertical coordinate along the cable, $t$ is the time, $L$ is the total cable length, and $J_0$ is the Bessel function of order zero.

For a 15 ft. cable or chain modelled by 13, 1 ft. links, Equation (32) was integrated for various $\theta_0$ and the results were compared with those produced by Equation (47). The comparisons of the predicted shapes as the chain passes the vertical are shown in Figures 4a. and 4b.

Natural Frequencies

For small oscillations, Woodward [14] has solved the governing partial differential equation of a hanging cable with a concentrated end mass. He has calculated and tabulated the natural frequencies for various end-mass to cable-mass ratios. By linearizing Equation (42), the analogous finite segment eigenvalue problem can be solved and the results compared with those of Woodward [14]. Table I. shows a comparison of results for the lowest frequency for chains of 5, 10, and 100 links respectively. Table II. shows the same comparison for the second lowest frequency. Finally, to show the convergence of the finite segment model, Table III. presents a comparison for the fifth frequency for chains of 5, 10, 20, 50, and 100 links. (The numbers listed in these tables are $2\omega_0 L/g$ where $\omega$ is the natural frequency.)
Table I. A Comparison of the Finite Segment Model and a Continuum Model for the Lowest Natural Frequency for a Hanging Cable.

<table>
<thead>
<tr>
<th>End-Mass/Cable-Mass Ratio</th>
<th>Number of Links</th>
<th>Continuum Results [14]</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>N</td>
<td>5</td>
</tr>
<tr>
<td>0.00</td>
<td>2.4077</td>
<td>2.405</td>
</tr>
<tr>
<td>0.1</td>
<td>2.317</td>
<td>2.315</td>
</tr>
<tr>
<td>0.2</td>
<td>2.261</td>
<td>2.260</td>
</tr>
<tr>
<td>0.5</td>
<td>2.174</td>
<td>2.173</td>
</tr>
<tr>
<td>1.0</td>
<td>2.113</td>
<td>2.113</td>
</tr>
<tr>
<td>2.0</td>
<td>2.067</td>
<td>2.067</td>
</tr>
<tr>
<td>3.0</td>
<td>2.030</td>
<td>2.030</td>
</tr>
<tr>
<td>10.0</td>
<td>2.016</td>
<td>2.016</td>
</tr>
<tr>
<td>20.0</td>
<td>2.008</td>
<td>2.008</td>
</tr>
<tr>
<td>50.0</td>
<td>2.003</td>
<td>2.003</td>
</tr>
</tbody>
</table>

Natural Frequency: $2^{-1/3}$
TABLE II. A Comparison of the Finite Segment Model and a Continuum Model for the Second Lowest Natural Frequency for a Hanging Cable.

<table>
<thead>
<tr>
<th>Number of Links</th>
<th>Continuum Results [14]</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>5</td>
</tr>
<tr>
<td>M/m_c</td>
<td></td>
</tr>
<tr>
<td>0.0</td>
<td>5.714</td>
</tr>
<tr>
<td>0.1</td>
<td>5.788</td>
</tr>
<tr>
<td>0.2</td>
<td>6.067</td>
</tr>
<tr>
<td>0.5</td>
<td>6.972</td>
</tr>
<tr>
<td>1.0</td>
<td>8.308</td>
</tr>
<tr>
<td>2.0</td>
<td>10.485</td>
</tr>
<tr>
<td>5.0</td>
<td>15.247</td>
</tr>
<tr>
<td>10.0</td>
<td>20.892</td>
</tr>
<tr>
<td>50.0</td>
<td>45.478</td>
</tr>
</tbody>
</table>

Natural Frequency $2\omega^2 L/3$
TABLE III. A Comparison of the Finite-Segment Model and a Continuum Model for the Fifth Natural Frequency for a Hanging Cable.

<table>
<thead>
<tr>
<th>N/Mc</th>
<th>5</th>
<th>10</th>
<th>20</th>
<th>50</th>
<th>100</th>
<th>Continuum Results [14]</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.0</td>
<td>22.632</td>
<td>16.965</td>
<td>15.584</td>
<td>15.065</td>
<td>14.968</td>
<td>14.931</td>
</tr>
<tr>
<td>0.1</td>
<td>24.392</td>
<td>19.192</td>
<td>18.028</td>
<td>17.686</td>
<td>17.636</td>
<td>17.619</td>
</tr>
<tr>
<td>0.5</td>
<td>30.719</td>
<td>26.223</td>
<td>24.931</td>
<td>24.564</td>
<td>24.512</td>
<td>24.493</td>
</tr>
<tr>
<td>1.0</td>
<td>37.291</td>
<td>32.567</td>
<td>31.014</td>
<td>30.578</td>
<td>30.515</td>
<td>30.493</td>
</tr>
<tr>
<td>2.0</td>
<td>47.904</td>
<td>42.300</td>
<td>40.311</td>
<td>39.756</td>
<td>39.675</td>
<td>39.648</td>
</tr>
<tr>
<td>5.0</td>
<td>70.871</td>
<td>62.869</td>
<td>59.928</td>
<td>59.112</td>
<td>58.996</td>
<td>58.951</td>
</tr>
<tr>
<td>10.0</td>
<td>97.861</td>
<td>112.282</td>
<td>82.820</td>
<td>81.679</td>
<td>81.546</td>
<td>81.466</td>
</tr>
<tr>
<td>20.0</td>
<td>136.722</td>
<td>121.384</td>
<td>115.709</td>
<td>114.146</td>
<td>113.953</td>
<td>113.811</td>
</tr>
<tr>
<td>50.0</td>
<td>214.691</td>
<td>190.418</td>
<td>181.655</td>
<td>179.264</td>
<td>178.654</td>
<td>178.511</td>
</tr>
</tbody>
</table>

Natural Frequency $2\omega_5 \frac{L}{\sqrt{g}}$
Submerged Catenary Cable

An experimental verification of the finite segment model can be obtained by comparison with data recorded at the U.S. Navy Civil Engineering Laboratory at Port Hueneme, California. In these experiments a totally submerged cable supported at one end, with a spherical body at the other end was initially held in a catenary shape and then released from rest [15,16]. The subsequent cable shape and motion were recorded. The same experiment was simulated using the finite segment cable model and the computer model SEADYN of Reference [15]. Table IV and Figure 5. show a comparison of the experimental and computed results for the end-body displacement. (In the analysis using Equations (32), the fluid forces were modelled by using results of References [3] and [17].)
TABLE IV. A Comparison of the Finite Segment Model, SEADYN [15,16] and Experimental Results for the End Displacement of a Submerged Cable.

<table>
<thead>
<tr>
<th>Time (s)</th>
<th>Experimental Results (X,Y)(ft)</th>
<th>SEADYN Results (X,Y)(ft)</th>
<th>Finite-Segment Model Results (X,Y)(ft)</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.0</td>
<td>20.5, 1.25</td>
<td>20.5, 1.25</td>
<td>20.5, 1.25</td>
</tr>
<tr>
<td>2.0</td>
<td>20.3, 6.65</td>
<td>20.71, 8.86</td>
<td>20.52, 7.97</td>
</tr>
<tr>
<td>4.0</td>
<td>20.2, 15.65</td>
<td>21.01, 16.81</td>
<td>21.11, 15.34</td>
</tr>
<tr>
<td>6.0</td>
<td>20.2, 24.15</td>
<td>21.31, 22.41</td>
<td>21.0, 22.21</td>
</tr>
<tr>
<td>8.0</td>
<td>20.5, 32.65</td>
<td>21.07, 27.47</td>
<td>20.89, 28.97</td>
</tr>
<tr>
<td>10.0</td>
<td>20.5, 40.55</td>
<td>20.77, 32.29</td>
<td>20.84, 35.78</td>
</tr>
<tr>
<td>12.0</td>
<td>20.5, 46.95</td>
<td>20.54, 37.35</td>
<td>20.88, 42.61</td>
</tr>
<tr>
<td>14.0</td>
<td>20.5, 51.15</td>
<td>20.24, 42.11</td>
<td>21.0, 49.19</td>
</tr>
<tr>
<td>16.0</td>
<td>20.0, 52.85</td>
<td>20.24, 46.81</td>
<td>21.05, 53.25</td>
</tr>
<tr>
<td>18.0</td>
<td>19.5, 53.55</td>
<td>19.64, 51.02</td>
<td>20.63, 54.06</td>
</tr>
<tr>
<td>20.0</td>
<td>18.9, 53.95</td>
<td>19.17, 53.07</td>
<td>19.89, 54.49</td>
</tr>
</tbody>
</table>

NOTE: X is the Horizontal Coordinate of the Spherical End-Body and Y is the Depth Coordinate.
Depth Prediction of Spherical End of Submerged Cable Released from Rest

![Graph showing depth prediction over time with experimental data points and model predictions.]

Figure 5. Comparison of Depth Predictions.
CONCLUSIONS AND APPLICATIONS

Figure 4a. shows excellent agreement between the finite segment model and the linear continuum model for small initial displacement angle. Indeed, the continuum model and the approximate solution of Equation (47) cannot be considered valid for large displacements. Figures 4a. and 4b. are thus in a sense a measure of the range of validity of the linear continuum model. Also, note that for large the finite segment model shows the period to be larger than that predicted by the linear model. This is consistent with nonlinear pendulum theory [18].

Tables I, II and III show the convergence of the finite-segment model to the linear continuum model for small amplitude oscillations. For the lowest frequency there is excellent agreement with only five links in the finite segment model. Moreover, even for the fifth frequency there is relatively good agreement with as few as ten links.

Finally, the comparison of the finite-segment model results with experimental results also shows good agreement. This comparison validates not only the finite-segment model, but also the modelling of the fluid forces as recorded in References [3] and [17].

These results all suggest that the finite-segment cable model can provide a very effective and efficient model of the nonlinear dynamic behavior of long heavy cables. Indeed the most appropriate applications are likely to be with long submerged towing cables, mooring cables, and hoisting cables.
Figure 4a. Comparison of Partial Differential Equations and Cable Model Results for Initial Amplitude Angles of 15°, 30°, and 45°.
Figure 4b. Comparison of Partial Differential Equation and Cable Model Results for Initial Amplitude Angles of 60°, 75°, and 90°.
REFERENCES


